GEOTECHNICAL EXPLORATION TECHNICAL MEMO

PREPARED FOR: Clinton Regional Development Corporation

PREPARED BY: Edward Schnackenberg, P.E. – Olsson Associates

Nathan Jensen, E.I. – Olsson Associates

PROJECT: Lincolnway Industrial Rail and Air Park

Clinton County, Iowa

DATE: February 10, 2016

OA PROJECT #: 015-2757

GEOTECHNICAL SCOPE

This memo was requested and authorized by Ms. Janelle Kreiling of the Clinton Regional Development Corporation for the purpose of evaluating the existing subsurface conditions for the Lincolnway Industrial Rail and Air Park in Clinton County, Iowa. The geotechnical work for this project included:

- Site reconnaissance and review of soil and geologic subsurface information from the USDA Natural Resource and Conservation Service (NRCS).
- Drilling and sampling of 9 soil test borings extending to depths ranging from 25 to 51.5 feet each below existing grades.
- Laboratory testing (as noted in the attachments) of soil samples obtained during the field operations.
- Completion of a geotechnical engineering evaluation using information obtained from our field observations, soil test borings, and laboratory testing program.
- Preparation of this memo presenting the soil test borings, laboratory test results, and a summary of our engineering evaluations and recommendations.

SITE LOCATION AND PROJECT DESCRIPTION

As proposed, the Clinton Regional Development Corporation plans to develop approximately 450 acres in Clinton County, Iowa bounded by Hwy 30 (Lincoln Highway) on the north, 44th Avenue on the south, 380th Avenue on the west, and 400th Avenue on the east. Refer to the attached *Site Location Plan* for more detailed property boundaries. A review of aerial photographs dating back to the mid 1930's indicate the project site has been and is currently utilized for row crop farming.

Topographic information obtained from the Clinton County Beacon database indicated the site has approximately 60 feet of grade change ranging from a high elevation of approximately 618 feet along Lincoln Highway at the north edge of the property to a low elevation of approximately 618 feet near a draw at the southeast corner.

Geotechnical recommendations, such as foundation design or settlement estimates, are not included in the scope of services for this preliminary exploration. When final site design, grading plans, and specific structures have been determined, final geotechnical exploration(s) will be required to provide earthwork and foundation recommendations for design and construction. More complicated structure designs or heavily loaded foundations may require a more detailed evaluation and geotechnical exploration. Olsson can provide a separate scope and fee for these future services if requested by the architect or developer.

FIELD EXPLORATION

A truck-mounted CME 75 drill rig was used to complete 9 soil borings for this project to depths ranging from 25 to 51.5 feet below existing grades. The soil boring locations and depths referenced in our proposal may have been shifted slightly in the field if it was necessary to avoid existing underground or overhead utilities, existing structures, site features, or areas of limited access. Refer to the attached *Boring Location Map* for the final boring locations. Boreholes were backfilled with native auger cuttings after completing the drilling operations. Ground surface elevations at each boring location were interpolated from the topographic survey provided by the Clinton County Beacon database.

Undisturbed and split-barrel soil samples were obtained at 2.5 to 5-foot depth intervals during the drilling process and returned to the laboratory for additional testing. Soil samples designated as "U" samples on the boring logs were obtained using thin-walled, steel Shelby tubes hydraulically pushed into the ground. Soil samples designated as "SS" samples on the boring logs were obtained with a split-barrel sampler. Recovered samples were sealed, labeled, and protected for transportation to our laboratory for testing.

LABORATORY TESTING

Descriptions of the soils encountered in the soil test borings were prepared in general accordance with ASTM D-2488 (Visual-Manual Procedure for Description and Identification of Soils). The soil stratification shown on the boring logs represents soil conditions at the specific boring locations, but variations may occur between or beyond the borings. The stratification lines represent the approximate boundaries between soil types, but the actual transitions between soil layers may be



gradual. Per the laboratory scope determined by Olsson engineers and the soil conditions encountered, tests were completed to evaluate the engineering properties of recovered soil samples.

AREA GEOLOGY AND SOIL STRATIGRAPHY

The USDA Soil Survey for Clinton County, Iowa considers the project site to be within a variety of soil complexes. Those are described as follows:

TABLE 1 - USDA Soil Survey	- Lincolnway Industi	rial Rail and Air Park,	, Clinton, IA
Map Unit Symbol & Soil	USCS Classification	Bedrock indicated	Bedrock
Name	OSCS Classification	(Y/N/NA)	depth (in.)
133 - Coly silt clay loam	CL, CH	N	
175B - Dickinson fine sandy	SC, SM, SP, and	N	
loam	variations	11	
184 - Klinger silt loam	ML, CL, & variations	N	
214C - Rockton loam	ML, CL, and	Υ	25 - 38
214C NOCKONIOUM	variations	'	23 30
217B - Ripon silt loam	ML, CL, SC, and	Υ	34 - 79
217b Riport Sitt Iodili	variations	'	34 73
284B - Flagler sandy loam	SC, SM, SP, and	N	
2045 Tragici Sariay loani	variations	14	
284D - Buckhardt sandy loam	SM, SC, SP, ML, and	N	
	variations	14	
350 & 350B - Waukegan silt	SM, SC, SP, ML, CL,	N	
loam	and variations	.,	
351 - Atterberry silt loam	SM, SC, ML, CL, and	N	
331 / Accesserry sile loans	variations	.,	
377B - Dunsdale silt loam	SM, SC, ML, CL, and	N	
	variations		
404 - Thorp silt loam	SM, SC, ML, CL, and	N	
	variations		
412D - Sogn Ioam	CL	Y	13 - 17
428B - Ely silt loam	CL, ML, CH, and	N	
.205 2., 6	variations		
760 - Ansgar silt loam	CL, ML, and	N	
	variations		
918 - Garwin silty clay loam	ML, CL, CH, MH, and	N	
	variations		
919 - Muscatine silt loam	ML, CL, CH, MH, SC,	N	
	SM, and variations		
	ML, CL, CH, MH, SC,		
920 & 920B - Tama silt loam	SM, SP, and	N	13 - 17
	variations		
	ML, CL, CH, MH, SC,		
1152 - Marshan clay loam	SM, SW, SP, and	N	
Note: The soil survey complex designs	variations		

Note: The soil survey complex designations and classifications are general in nature and represent existing soil conditions to depths of up to 80 inches only. The boundaries shown on the accompanying map are not definite and variations between unit designations and soil classifications should be anticipated.



The wide variation in soil complex descriptions can be generally summarized as silts and clays with variations in sand content, all overlying glacial drift. The original surface geology has been altered during seasonal row crop farming practices.

Of special note in Table 1 are the map unit designations for 214C, 217B, and 412D referencing the presence of limestone bedrock within 13 to 79 inches of the ground surface. For reference, these general areas are hatched on the attached *USDA Soil Survey Map*. Although final geotechnical explorations should confirm or deny the presence of bedrock in these areas, we have included the potential shallow bedrock locations in this report for consideration during future site development. In most instances, other than increased earthwork and demolition costs, shallow bedrock should not pose a significant concern for overall site development.

A review of the Iowa Department of Natural Resources (IDNR) Coal Mine database indicated the closest coal mine or prospect holes are located approximately 29.5 miles south/southwest of the project site. Refer to the attached *IDNR Coal Mine Map* for additional details.

SOIL PROPERTIES - BORING LOCATIONS SAMPLED

Loess – Soft to very stiff, light brown to dark grayish brown, slightly moist to very moist, fat and lean clay.										
USCS Classification	Dry Density (pcf)	Moisture Content (%)	Saturation (%)	LL/PI (%)	P-200 (%)	Unconfined Strength (tsf) (SPT (blows/ft))				
CH, CL	88.8 – 105.5	14.5 – 33.1	65.5 – SAT.	67/19	N/A	1.1 (3 – 4)				

N/A-Not Applicable

Glacial Drift (Cohesive) – Firm to hard, light brown to dark gray to bluish grayish, slightly moist to wet, lean clay with varying sand and gravel content.

USCS Classification	Dry Density (pcf)	Moisture Content (%)	Saturation (%)	LL/PI (%)	Unconfined Strength (tsf) (SPT (blows/ft))
CL	88.6 – 127.4	8.3 – 39.7	60.3 – SAT.	29-41/12-16	1.0 (6 – 50/2")

N/A-Not Applicable

Glacial Drift (Cohesionless) – Very loose to dense, light brown to reddish brown to dark gray, slightly moist to wet, sands with varying fines and gravel contents.

USCS Classification	Dry Density (pcf)	Moisture Content (%)	Saturation (%)	P-200 (%)	Unconfined Strength (tsf) (SPT (blows/ft))
SC, SP-SC, SP	113.8 – 127.5	6.9 – 21.5	93.0 – SAT.	3.5 – 49.9	0.6 – 1.1 (3 – 50/0.5")



GROUNDWATER SUMMARY

Free water was observed in all 9 of the soil borings completed during this exploration. Groundwater measurements obtained during this exploration are presented in the following table. Groundwater and drainage considerations for future development are presented in the *Drainage and Groundwater Considerations* section of this memo.

GROUNDWATER MEASUREMENTS

Boring No.	Groundwater Depth While Drilling (ft)	Groundwater Elevation While Drilling (ft)	Groundwater Depth Immediately After Drilling (ft)	Groundwater Elevation Immediately After Drilling (ft)
B-1	5.5	628.5	5.0	629.0
B-2	8.0	652.0	7.5	652.5
B-3	5.0	630.0	4.5*	630.5
B-4	5.0	645.0	3.5	646.5
B-5	6.0	666.0	4.0*	668.0
B-6	6.0	667.0	10.5	662.5
B-7	5.0	641.0	20.7	625.3
B-8	18.0	646.0	14.9	649.1
B-9	8.0	638.0	6.5	639.5

^{*-}Groundwater measurement taken at least 10 hours but less than 24 hours after completion of soil boring

It should be noted that groundwater levels (perched or otherwise) typically fluctuate with seasonal variations in precipitation, drainage, runoff, snowmelt, irrigation demands, and other factors that may differ from those at the time of drilling operations.

SITE PREPARATION

At the time of drilling, the project site was covered with recently harvested crop stubble and shallow weeds and grasses. Prior to site grading, vegetation, trees (including the root balls), frozen soil, and other deleterious or unsuitable materials should be stripped and removed from areas of new construction. We anticipate stripping depths on the order of 6 to 10 inches will be required to remove old crop stubble and root crowns, but isolated areas may require stripping to slightly greater depths. If requested, an Olsson field representative can be contracted to help determine final stripping or removal depths in areas of concern. If encountered, known or unknown structures, including foundations, floor slabs, and basement walls or floors should be completely removed and the resulting excavations replaced with structural fill. Topsoil removed during the stripping operations should not be reused as structural fill below or around new



structures, pavements, or for utility backfill but may be reused in landscaped or other non-loaded areas around the project site.

Site clearing, grubbing, and stripping should be completed during periods of dry weather. Operating heavy equipment on the site during periods of wet weather could result in excessive pumping and rutting of the subgrade soils. The base of new construction excavations should be evaluated by an Olsson geotechnical engineer or their authorized representative prior to the placement of new fill soils. New structural fill should be placed and compacted in accordance with the recommendations presented in the *Structural Fill* section of this memo.

In areas requiring new structural fill, the contractor should lightly scarify the exposed subgrade, moisture condition as necessary, and compact the subgrade soils in accordance with the recommendations in the *Structural Fill* section of this memo. After preparation and compaction, areas to receive new structural fill should be proofrolled with a loaded tandem-axle dump truck, scraper, or similar rubber-tired equipment weighing at least 25 tons. Proofrolling operations should be observed and documented by an Olsson field representative. Unstable or unsuitable soils which are revealed by proofrolling and which cannot be reworked, moisture conditioned, and adequately compacted in-place should be documented, removed, and replaced with new compacted structural fill or be stabilized in accordance with the *Structural Fill* section of this memo. The geotechnical engineer should be contacted if additional subgrade stabilization is required to prepare the site for construction.

During this exploration, our soil borings identified 2.5 to 6.0 feet (includes the agriculture zone thickness) of expansive (fat) clay loess. These surface materials varied from moist to very moist and soft to very stiff at the locations sampled. During future development, areas requiring cut may remove these surface clays and expose the underlying sands and sands with clay. If or where this occurs, the native sands may be susceptible to increased erosion and rutting under earthwork equipment traffic. In some areas, it may be possible to improve erosion resistance and increase subgrade stability by incorporating the on-site clays into the exposed sands. A 30 to 50 percent blend ratio would be considered typical but should be determined in the field at the time of earthwork. The geotechnical engineer can be contracted to assist with this process if desired.

SUBGRADE STABILIZATION (IF REQUIRED)

If areas of the project site expose very moist or unstable soils, it may be feasible to scarify the subgrade soils, allow them to dry to near optimum moisture, and compact them following the recommendations of this memo. If unstable conditions persist and additional subgrade



stabilization is necessary, 3- to 4-inch thick lifts of crushed aggregates (2- to 3-inch diameter top-sized particles) could be driven into the exposed subgrade, using an appropriately sized sheepfoot roller (no vibration), until stable. Initially, this stabilization method should be attempted in an isolated "test pad" area to determine its effectiveness using the available materials and the contractor's means and methods. The "test pad" approach will limit costs until the effectiveness of the applicable stabilization method can be documented in the field. In the case of extremely soft or unstable subgrades, the use of geosynthetic fabric and/or geogrid below the crushed aggregates may be more efficient and cost effective by reducing the overall aggregate thickness. Well graded, crushed limestone or crushed recycled concrete with maximum top sized particles of 1½ inches are recommended when stabilization methods include the use of geogrid. If desired, an Olsson geotechnical engineer can be contracted to evaluate subgrade conditions at the time of earthwork and provide more specific recommendations for stabilization using geosynthetics and crushed aggregate.

Subgrade stabilization may also be necessary at the base of utility trenches across low-lying areas of the project site. The methods of subgrade stabilization described previously also apply to the base of utility trenches; however, a backhoe-mounted, sheepfoot type, trench roller would be the preferred method of compaction for stabilization aggregates and cohesive backfill soils in this application.

It is the responsibility of the earthwork contractor to utilize equipment and procedures that prevent unnecessary deterioration or damage to exposed subgrade soils. It may be necessary to utilize low ground pressure (LGP) equipment in low lying areas or in natural exposed sands that will minimize disturbance of very moist subgrade soils during excavation. Heavy, rubber-tired construction equipment may not be suitable for use in low lying areas of this site, as this equipment is more likely to disturb potentially sensitive subgrade soils. The contractor should provide a uniform and stable soil subgrade as part of the final grading operations. Unstable soil subgrade or instability related to repetitive construction traffic is the responsibility of the contractor to repair or replace at no additional cost to the owner. If unstable soil conditions are encountered across the project site, the geotechnical engineer should evaluate and document these unsuitable conditions and will recommend appropriate corrective action for removal and replacement or inplace stabilization.



STRUCTURAL FILL

During earthwork and construction, we recommend that on-site and imported fill materials have a liquid limit less than 50 and a plasticity index less than 25. Soils which have a liquid limit greater than 50 and a plasticity index greater than 25 typically require blending with less plastic materials to reduce the expansive characteristics. In addition to maintaining the recommended plasticity criteria, the fill soils should be relatively free of organic materials (less than about 2 percent by weight) or other deleterious materials and should not contain particles larger than 2 inches. Based on laboratory test results, the surface soils sampled during this exploration are not within the previously mentioned soil parameters. In this instance, reuse of on-site soils as structural fill may be acceptable in areas of deeper fill. During the final geotechnical explorations prior to future development, the use of blended on-site soils (to reduce plasticity), imported low plastic cohesive fill, or soil amendments such as lime or fly ash may be recommended to provide a low plastic separation layer between future building slabs and pavement and the underlying moisture sensitive expansive soils. All new structural fill should be free of debris and excessive organic material and should be properly moisture conditioned prior to compaction.

Suitable fill materials should be placed in thin lifts. Lift thickness depends on the type of compaction equipment, but in general, lifts of 4 to 8-inch loose measurements are recommended. The soil should be compacted using appropriately sized equipment capable of achieving the compaction recommendations of this memo. A self-propelled sheepfoot roller, such as CAT 815, is generally recommended for compacting cohesive soils over large areas. The contractor should take care if working near existing structures or site features, and it may be necessary to restrict or eliminate the use of vibration to prevent damage. Within small excavations, such as in footing trenches, utility trenches, or around manholes, we recommend the use of "Wacker-Packers" or "Rammax" compactors for cohesive soils or vibrating plate compactors for granular soils to achieve the specified compaction. Lift thicknesses should be reduced to 4 inches in small fill areas requiring hand-operated equipment.

During grading operations, representative samples of general and structural fill materials should be initially and periodically checked by laboratory testing to document that the previously mentioned soil parameters are maintained. An Olsson representative should regularly observe and monitor excavation and grading operations and perform field density tests to document that the specified moisture and compaction requirements are being achieved. We recommend that general fill, structural fill, and utility backfill be compacted and moisture conditioned in accordance with the following table:



FILL PLACEMENT/COMPACTION GUIDELINES

Areas of Fill Placement	Compaction Recommendation (ASTM D-698 Standard Proctor)	Moisture Content (Percent of Optimum)
Structural Fill – Cohesive fill soils placed during mass grading operations.	95%	-2 to +3 percent
Structural Fill – Granular fill soils placed during mass grading operations.	95%	As necessary to achieve compaction
Non-Structural Fill – Beneath non-loaded landscape/grass areas	92%	As necessary to achieve compaction

The moisture content of imported or on-site soils at the time of compaction should generally be maintained between the ranges specified above. More stringent moisture limits may be necessary with certain soils, and some adjustments to moisture contents may be necessary to achieve compaction in accordance with project specifications.

DRAINAGE AND GROUNDWATER CONSIDERATIONS

During future site development, free or perched groundwater and saturated soil conditions could adversely impact site grading, earthwork, or building construction in low lying areas of the project site. If extensive cut is proposed in areas with shallow groundwater, temporary or permanent dewatering procedures may be necessary. Information pertaining to the need for dewatering should be provided during the final geotechnical exploration(s) after site grading requirements and structure designs have been determined.

Surface water or precipitation should not be allowed to collect at the ground surfaces either during or after construction. Provisions should be made to quickly remove accumulating seepage or storm water runoff from excavations. Undercut or excavated areas should be sloped toward one corner to allow rainwater or surface runoff to be quickly collected and gravity drained or pumped from construction areas. Subgrade soils that are exposed to precipitation or runoff should be evaluated by the geotechnical engineer prior to the placement of new fill to determine if corrective action is required.

With shallow groundwater indicated across most of this site, provisions for rapid and efficient drainage should be anticipated during future site development. This could include trench drains below or around parking lots or roadways, perimeter drains around shallow building foundations, and properly designed and constructed surface grading which promotes positive drainage away from new structures or site features.



CONSTRUCTION EQUIPMENT MOBILITY

On-site soils or imported structural fill may be susceptible to softening under construction equipment traffic during periods of wet weather. Reducing equipment mobility problems and managing soft surface soils will be dependent on the severity of the circumstances, the soil types, the season in which construction is performed, and prevailing weather conditions.

Some general guidelines for reducing equipment mobility problems and addressing potential soft and wet surface soils are as follows:

- Optimize surface water drainage at the site during construction.
- Whenever possible, wait for dry weather conditions to prevail, and do not operate construction equipment on the site during wet conditions. Rutting the surface soils will aggravate the condition and accelerate subgrade disturbance.
- Disk or scarify wet surface soils during favorable weather to accelerate drying.
 Temporarily compact loose subgrade soils if rain is forecast to promote site drainage and reduce moisture infiltration.
- Use construction equipment that is well-suited for the intended job under the existing site
 conditions. Heavy rubber-tired equipment typically requires better site conditions than
 light, track-mounted equipment.
- Implement a construction schedule that realistically allows for rain days. Pressure to perform earthwork under a tight schedule is frequently counterproductive.

If requested, Olsson engineers can help determine the best approach for stabilizing unsuitable soils at the time of construction.

TEMPORARY SLOPES AND EXCAVATIONS

Construction site safety is the responsibility of the general contractor. The contractor shall also be solely responsible for the means, methods, techniques, sequencing, and operations during construction. Olsson is providing the following information solely as a service to our client. Under no circumstances should Olsson's provision of the following information be construed to mean that we are assuming responsibility for construction site safety or the contractor's activities. Such responsibility is not implied and should not be inferred.

The contractor should be aware that slope height, slope inclination, and excavation depths (including utility trench excavations) should in no case exceed those specified in local, state, or federal safety regulation; e.g., OSHA Health and Safety Standards for Excavations, 29 CFR Part 1926, or successor regulations. Such regulations are strictly enforced, and if not followed, the



owner, the contractor, or earthwork or utility subcontractors could be liable for substantial penalties.

SEISMIC CLASSIFICATION

According to the International Building Code (IBC), soils within the upper 100 feet determine the seismic classification criteria for the project site. Based on the soils encountered in our test borings and our experience with the local geology, Olsson estimated the soil properties below the deepest boring to a depth of 100 feet. The soil shear strengths and N values were estimated based on the results of the laboratory testing program and the assumed properties of the undocumented soils and rock below the lowest boring. For this project site, we recommend using Site Class C (Very Dense Soil and Soft Rock) according to Section 1613 of the 2012 IBC.



LIMITATIONS

The conclusions presented in this memo report are based on the information available regarding future development, the results obtained from our soil test borings and sampling procedures, the results of the laboratory testing program, and our experience with similar projects. The soil test borings represent a very small statistical sampling of subsurface soils, and it is possible that conditions may be encountered during construction that are substantially different from those indicated by the soil test borings. In these instances, adjustments to design and construction may be necessary. This geotechnical memo is based on the initial information provided to Olsson and our understanding of the project as noted in this memo.

This memo was prepared under the direction and supervision of a Professional Engineer registered in the State of Iowa with the firm of Olsson Associates. The information contained herein is based on generally accepted, professional geotechnical engineering practices at the time of this memo, within this geographic area. No warranty, express or implied, is intended or made. This memo has been prepared for the exclusive use of Clinton Regional Development Corporation for specific application to the proposed project. Olsson appreciates the opportunity to provide our services on this project and looks forward to working with you during the final geotechnical exploration. Should you have any questions, please do not hesitate to contact us.

Respectfully submitted, *Olsson Associates*

Nathan Jensen, E.I. Assistant Engineer

Attachments:
Site Location Plan
Boring Location Map
Summary of Laboratory Results
USGS Soil Survey Map
USGS Soil Survey Test
IDNR Coal Mine Map

EDWARD M. Q. W. SCHNACKENBERG M. 18218
2/10/10

Edward Schnackenberg, P.E. Geotechnical Engineer











SITE LOCATION PLAN

LINCOLNWAY INDUSTRIAL RAIL AND AIR PARK
NEAR LINCOLN HWY (US HWY 30) AND COUNTY HWY Z40
CLINTON COUNTY, IOWA
OLSSON PROJECT NO. 015-2757



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	015-	2757				Clinton County, Iowa							T
ELEVATION (ft)	Shelby Tube	Split Spoon		SRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	LL/PI (%)	ADDITIONAL DATA/
ELE	MATERIAL DI	ESCRIPTION		GR.	B	SAMP	LASSI	BLO N-V	OND	MOM	DRY (REMARKS
		ACE ELEV. (ft): 634			0		0						
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	LOESS		1.0'							\vdash		-	
	- Fat clay (CH), stiff, moist organics	t, grayish brown with			 _	1 1			1.1	22.9	99.8	_	
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_ 630 _	GLACIAL DRIFT		4.0'		_ 5	U 2				17.3			
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625	Clayey sand (SC), mediu	ım dense. wet.						5-6-7		40.0			D 000 05 00/
	yellowish brown with iron	staining			10	4		N=13		18.9			P-200 = 25.9%
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010	Poorly graded sand with					SS 6		5-13-14 N=27		18.8			P-200 = 3.5%
	dense, wet, light grayish yellow		20.0	•	20	/ \ \		IN-21		\perp			
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WAT	ER LEVEL OBSERVATIONS							STARTE)	1/24	4/16 FI	NISHEI	D 1/24/16
WD	∑ 5.5 ft	OLSSON						DRILL CO)		OA DI	RILL RI	G CME 75
IAD	▼ 5.0 ft	8720 SOUTH 114					107						
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METHOD

CONTINUOUS FLIGHT AUGER

Not Performed

OLSSON ®
PROJECT NAME
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LOG OF BOREHOLE NO. B-1

Sheet 2 of 2

	ASSOCIATES										Sheet 2 of 2
PRO	JECT NAME	·	CLIE	NT							
	Lincolnway Industrial Rail and Air Park	(Clinton Regional Development Corporation				ation				
PRO	JECT NUMBER		LOC	ATION							
	015-2757					Clintor	ı Coı	ınty,	Iowa		
N O	Shelby Tube Split Spoon	ပ	_	TYPE	ATION 3)	.,e.,	αż	ZE) TIIS		
ELEVATION (ft)	MATERIAL DESCRIPTION	GRAPHIC	20 DEPTH	SAMPLE TYF NUMBER	CLASSIFICA (USCS	BLOWS/ N-VALU RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS
	GLACIAL DRIFT		 								
610	Clayey sand with gravel (SC), loose, wet, yellowish brown mottled with gray	25.0'	 25 /	SS 7		2-3-3 N=6					

BASE OF BORING AT 25.0 FEET

WAT	WATER LEVEL OBSERVATIONS									
WD	∑ 5.5 ft									
IAD	▼ 5.0 ft									
AD	▼ Not Performed									

STARTED	1/24/16	FINISHED	1/24/16
DRILL CO.	OA	DRILL RIG	CME 75
DRILLER	JY	LOGGED BY	SS
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PRO	JECT NAME Lincolnway Industrial	Rail and Air Park	(CLII		ton R	egional	Deve	lopm	ent C	orpor	ation
PRO	JECT NUMBER 015-27	57			LOC	CATION		Clinto	n County, Iowa				
ELEVATION (ft)	Shelby Tube No Recovery MATERIAL DESC	Split Spoon		GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR.			(%)	ADDITIONAL DATA/ REMARKS
660	APPROX. SURFACI	E ELEV. (ft): 660	0.5'		0								
	LOESS		0.5										
	Fat clay (CH), stiff, very mo with organics	ist, grayish brown	2.5'			U 1	СН			26.3	95.4	67/19	
	GLACIAL DRIFT												
 _ 655	Clayey sand with gravel (SC slightly moist, reddish browi	c), medium dense, n with iron staining			 5	SS 2		8-10-11 N=21		14.8		_	
	Clayey sand with gravel (SC slightly moist, orangish brov					SS 3		1-1-2 N=3		14.6			
_	. ☑ Clayey sand with gravel (SC grayish brown with iron stail	C), loose, wet,				SS 4		2-2-5 N=7		15.1			P-200 = 34.3%
			45.0		45	○ NR 5							
645	Sandy lean clay (CL), stiff, v	vet, brown with iron	15.0		15 	SS 5		4-5-8 N=13		13.6			

METHOD

1/24/16

CME 75

CONTINUOUS FLIGHT AUGER

SS

3-5-7 N=12 SS Sandy lean clay (CL), stiff, wet, dark gray with 17.8 iron staining 640 20.0' 20 **CONTINUED NEXT PAGE** WATER LEVEL OBSERVATIONS STARTED 1/24/16 FINISHED **OLSSON ASSOCIATES** WD DRILL CO. OA DRILL RIG 8720 SOUTH 114TH STREET, SUITE 107 ▼ 7.5 ft IAD **LA VISTA, NE 68128** DRILLER JY LOGGED BY

Value of the second of the

AD

								•				
	OLSSON ®	LOG OF B	ORE	HOL	E NC). B	-2				5	Sheet 2 of 2
PRO	JECT NAME Lincolnway Industrial	Rail and Air Park		CLI		ton R	egional l	Deve	lopm	ent C	orpor	ation
PRO	JECT NUMBER 015-27	57		LOC	CATION		Clinto	ı Coı	ınty,	lowa		
elevation (ft)	Shelby Tube No Recovery MATERIAL DESC	Split Spoon CRIPTION	GRAPHIC LOG	OS DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS
	GLACIAL DRIFT											

BASE OF BORING AT 25.0 FEET

Sandy lean clay (CL), stiff, wet, light brown mottled with gray and iron staining

635

WAT	ER LEVEL OBSERVATIONS
WD	∑ 8.0 ft
IAD	▼ 7.5 ft
AD	▼ Not Performed

OLSSON ASSOCIATES 8720 SOUTH 114TH STREET, SUITE 107 LA VISTA, NE 68128

STARTED	1/24/16	FINISHED	1/24/16
DRILL CO.	OA	DRILL RIG	CME 75
DRILLER	JY	LOGGED BY	SS
METHOD	CONTINUO	US FLIGHT AU	GER

13.5 105.2

	OLSSON ®	LOG OF BO	RE	HOL	E NC). B	-3	Sheet 1 of 3					
PRO	JECT NAME Lincolnway Industrial	Rail and Air Park		CLII		ton R	egional I	Deve	lopm	ent C	orpor	ation	
PRO	JECT NUMBER 015-27			LOC	CATION	Clinton County, Iowa							
	Shelby Tube	Split Spoon				z	Cilitor	000	irity,				
ELEVATION (ft)	MATERIAL DESC	CRIPTION	GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	LL/PI (%)	ADDITIONAL DATA/ REMARKS	
635	APPROX. SURFACE AGRICULTURAL ZONE	E ELEV. (ft): 635		0									
		1.0'											
	LOESS				U 1				22.9	105.1			
	Fat clay (CH), very stiff, mod brown with organics												
630	Ţ GLACIAL DRIFT	4.0'		 5	U 2				12.8				
	Poorly graded sand with cla medium dense, wet, reddish	ov (SP-SC)			<u> </u>								
	staining				SS 3		4-5-5 N=10		20.6				
					\ /								
_ 625	Poorly graded sand with cla wet, grayish brown	y (SP-SC), loose,		10	SS 4		1-2-2 N=4		19.3			P-200 = 9.6%	
		13.0°											
					\ /								
 620	Clayey sand (SC), loose, we	et, light brown		15	SS 5		4-3-5 N=8						
-													
	Clayey sand (SC), medium brown	-			SS 6		3-5-11 N=16						
615	CONTINUED N	20.0'		20	/ \ <u> </u>								
WAT	ER LEVEL OBSERVATIONS						STARTED)	1/22) _{/16} FII	NISHEI) 1/22/16	
WD	∑ 5.0 ft	OLSSON AS	SOC	ATE	S	40-	DRILL CO				RILL RI		

8720 SOUTH 114TH STREET, SUITE 107

LA VISTA, NE 68128

DRILLER

METHOD

JY LOGGED BY

HOLLOW STEM AUGER

SS

▼ Not Performed

IAD

AD

	OLSSON ®	LOG OF BO	RE	HOL	E NC). B	-3	Sheet 2				
PRO	JECT NAME Lincolnway Industri	ial Rail and Air Park		CLII		ton R	egional I	Deve	lopm	ent C		
PRO	JECT NUMBER			LOC	CATION						p	
	015-2	2757				1	Clintor	ı Coı	ınty,	lowa		
ELEVATION (ft)	Shelby Tube MATERIAL DE	Split Spoon ESCRIPTION	GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS
			g		SAN	CLAS	⊠ Z	5	M	DR		
615	GLACIAL DRIFT			20								
	Lean clay (CL), firm, wet,	dark gray			SS 7	CL	2-3-4 N=7		36.5		41/16	
610				25	/ V						_	
 605	Lean clay (CL), firm, wet,	dark gray		 30	SS 8		2-3-4 N=7		39.7			
					U 9				38.2	88.6		
	Lean clay (CL), firm, wet,	dark gray		 35 _	U 10				38.7			
 595	Sandy lean clay (CL), stil mottled with gray	ff, wet, light brown 40.0 '		 40	U 11				21.6	98.0		
	CONTINUED	NEXT PAGE	(////									
WAT	ER LEVEL OBSERVATIONS						STARTED)	1/22	2/16 FII	NISHEI	D 1/22/16
WD	∑ 5.0 ft	OLSSON ASS					DRILL CC).		OA DF	RILL RI	G CME 75
IAD	▼ Not Performed	8720 SOUTH 114TH S LA VISTA, N				10/	DRILLER			JY LC	GGED	BY SS

METHOD

HOLLOW STEM AUGER

 Ψ 4.5 ft after 24 Hrs

AD



LOG OF BOREHOLE NO. B-3

Sheet 3 of 3

	ASSOCIATES										Sheet 3 of 3
PRO	JECT NAME		CLIENT								
	Lincolnway Industrial Rail and Air Park	(Clinton Regional Development Corporation								
PRO	JECT NUMBER		LOC	ATION			_				
	015-2757					Clinto	1 Col	ınty,	Iowa	I	
NOI	Shelby Tube Split Spoon	SHC	,	TYPE ER	S)	3/6" .UE	표 _	JRE	VSITY (_	ADDITIONAL
ELEVATION (ft)	MATERIAL DESCRIPTION	GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	DATA/ REMARKS
595			40		0						
 590	GLACIAL DRIFT Sandy lean clay with gravel (CL), very stiff, wet, bluish gray		45	U 12				17.8			
	Sandy lean clay with gravel (CL), very stiff, wet, bluish gray	51.5	50	U 13 SS 13		6-7-13 N=20		13.7	127.4		
	BASE OF BORING AT 51 5 FEFT										

BASE OF BORING AT 51.5 FEET

WAT	ER LEVEL OBSERVATIONS
WD	☑ 5.0 ft
IAD	▼ Not Performed
AD	

STARTED	1/22/16	FINISHED	1/22/16
DRILL CO.	OA	DRILL RIG	CME 75
DRILLER	JY	LOGGED BY	SS
METHOD	HOLLOW S	TEM AUGER	

	OLSSON ®	LOG OF BO	RE	HOL	E NC). B	3-4				5	Sheet 1 of 2
PRO	JECT NAME Lincolnway Industr	ial Rail and Air Park		CLII		ton R	egional	Dovo	lonm	ont C		
PRO	JECT NUMBER			LOC	CATION	101111					oi poi	ation
	Shelby Tube	Split Spoon				z	Clinto	COL	unty,			
ELEVATION (ft)	MATERIAL DI		GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	LL/PI (%)	ADDITIONAL DATA/ REMARKS
650	APPROX. SURFA AGRICULTURAL ZONE	ACE ELEV. (ft): 650		0		0						
	LOESS	0.5										
	Fat clay (CH), stiff, very i brown with organics				U 1				28.4	91.2		
-	▼ GLACIAL DRIFT	3.0'										
 645	Clayey sand with gravel reddish brown	(SC), loose, wet,		5	SS 2		4-2-6 N=8		16.7			
	Clayey sand with gravel grayish brown	(SC), loose, wet,			SS 3		2-3-4 N=7		11.5			P-200 = 36.5%
 _ 640	Clayey sand with gravel gray	(SC), loose, wet, dark		10	SS 4		3-3-3 N=6		37.3			
 		12.5			\							
_ 635 	Sandy lean clay with gra dark gray	vel (CL), firm, wet,		15	SS 5		1-2-4 N=6		18.3			
 630	Sandy lean clay (CL), sti	20.0	,	20	SS 6		3-4-7 N=11		16.5			
\A/A T		NEXT PAGE										
WAI	FER LEVEL OBSERVATIONS $\ \ \ \ \ \ \ \ \ \ \ \ \ $	OLSSON AS	SOCI	ATE	S		STARTE				NISHE	
IAD	型 3.5 ft	8720 SOUTH 114TH S LA VISTA,	STRE	ET, S	SUITE '	107	DRILL CO				RILL RI DGGED	
I		LA VIOIA,		J . <u> </u>								. 00

▼ Not Performed

CONTINUOUS FLIGHT AUGER

SS

METHOD



Sheet 2 of 2

PROJECT NAME Lincolnway Industrial Rail and Air Park PROJECT NUMBER O15-2757 Shelby Tube Shelby Tube Shelby Tube Shelby Tube MATERIAL DESCRIPTION Clinton County, Iowa Shelby Tube Shelby Tube MATERIAL DESCRIPTION Shelby Tube Sh		ASSOCIATES										•	Sneet 2 of 2
PROJECT NUMBER O15-2757 Clinton County, lowa Shelby Tube Shelby	PRO	JECT NAME			CLIE	NT							
Clinton County, lowa Shelby Tube Shelby Tube Shelby Tube MATERIAL DESCRIPTION Samble LAA DATA REMARKS (st) Clinton County, lowa ADDITIONAL BRAY CROP CROSS OR Samble LAA ADDITIONAL DATA REMARKS Clinton County, lowa ADDITIONAL DATA REMARKS Samble LAA ADDITIONAL DATA REMARKS SS Samble LAA SS Sandy lean clay (CL), stiff, wet, dark gray SS To S-6-8 N=14 SS To S-6-8 N=14		Lincolnway Industrial	Rail and Air Park			Clin	ton Re	egional [Deve	lopm	ent C	orpor	ation
Shelby Tube Shelb	PRO	JECT NUMBER			LOC	ATION							
MATERIAL DESCRIPTION ABOUT ABOUT		015-275	57					Clintor	ı Coı	ınty,	lowa		
MATERIAL DESCRIPTION ABOUT ABOUT	NO	Shelby Tube	Split Spoon	ပ		Y PE	TION	., ш	ر من	₹.	ПТ		
GLACIAL DRIFT		MATERIAL DESC	RIPTION	GRAPHI LOG		SAMPLE T NUMBEI	CLASSIFICA (USCS)	BLOWS/ N-VALU RQD	UNC. STF (tsf)	MOISTUR (%)	DRY DENS (pcf)	LL/PI (%)	DATA/
Sandy lean clay (CL), stiff, wet, dark gray SS 7 5-6-8 N=14 15.5	630	CLACIAL DRIET		////	20								
N=14 19.5		GEAGIAE DIGIT											
N=14 19.5													
N=14 19.5													
N=14 19.5													
					k	4							
1 625 25 0 √/// 25 / \		Sandy lean clay (CL), stiff, w	et, dark gray							15.5			
PASE OF PODING AT 25 O FEET	625			25.0'	25	<u>/ \</u>							

BASE OF BORING AT 25.0 FEET

WAT	ER LEVEL OBSERVATIONS
WD	∑ 5.0 ft
IAD	▼ 3.5 ft
AD	▼ Not Performed

STARTED	1/24/16	FINISHED	1/24/16
DRILL CO.	OA	DRILL RIG	CME 75
DRILLER	JY	LOGGED BY	SS
METHOD	CONTINUO	US FLIGHT AU	GER

	OLSSON ®	LOG OF	BOR	EH	IOL	E NC). В	-5				5	Sheet 1 of 3	
PRO	JECT NAME Lincolnway Industr	ial Rail and Δir Park			CLIE		ton R	egional l	Deve	lonm	ent C			
PRO	JECT NUMBER				LOC	ATION						oi poi	ution	
	015-	2757		\perp				Clintor	ı Coı	ınty,	lowa		I	
ELEVATION (ft)	Shelby Tube	Split Spoon	PHIC	LOG	(ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	LL/PI (%)	ADDITIONAL DATA/	
ELEV	MATERIAL DI	ESCRIPTION	GR/	-	5	SAMPI	LASSI (U	BLO N-N R	UNC	MOM	DRY D		REMARKS	
		ACE ELEV. (ft): 672			0		O							
	AGRICULTURAL ZONE		1.0'											
	LOESS		1.0		1									
670	Fat clay (CH), stiff, very i brown with organics	moist, dark grayish			-	1 1				33.1	93.0	_		
	GLACIAL DRIFT		3.0'		1									
	Clayey sand (SC), media reddish brown with iron s	um dense, wet, staining			5	U 2				15.5				
	. ⊻			/ -	+	/								
665	Clayey sand with gravel wet, light grayish brown	(SC), medium dense,				SS 3		11-10-12 N=22		11.3			P-200 = 22.9%	
	Driller's Note: Auger cha Line at 7.5 feet	atter indicated Stone				,								
	Ente di 7.0 foci		9.0'		7	. /								
			_9.0		10	SS 4		7-2-1 N=3		11.6				
					10	V						_		
					4									
660														
					1									
					4									
	Sandy lean clay with gra light brown with iron stair	vel (CL), stiff, wet, ning			_]	SS 5		4-6-8 N=14		14.4				
					15 /	<u> </u>						_		
655														
_ 000					-									
					4									
	Sandy lean clay (CL), ve brown with iron staining	ry stiff, wet, grayish	20.0'		- 20	U 6				16.4	119.5			
	CONTINUED	NEXT PAGE	20.0	′//										
WAT	ER LEVEL OBSERVATIONS		'	,	1			STARTED)	1/23	3/16 FII	VISHE	D 1/23/16	
WD	WD ✓ 6.0 ft OLSSON ASS							DRILL CO				DRILL RIG CME 75		
IAD	▼ Not Performed	8720 SOUTH 114 LA VIS				UITE '	107	DRILLER				GGED		
.			,	. 55							-0		50	

METHOD

HOLLOW STEM AUGER

SS

AD

▼ 4.0 ft after 14 Hrs

	OLSSON ®	LOG OF BO	RE	HOL	E NC). B	-5		Sheet 2 o				
PRO	JECT NAME Lincolnway Industr	al Rail and Air Park		CLIE		ton R	egional I	Deve	lopm	ent C			
PRO	JECT NUMBER			LOC	CATION								
	015-	2757					Clintor	ı Coı	ınty,	lowa			
ELEVATION (ft)	Shelby Tube MATERIAL DI	Split Spoon ESCRIPTION	GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	LL/PI (%)	ADDITIONAL DATA/ REMARKS	
			////	20		ਹ							
 _650 	GLACIAL DRIFT			 									
	Sandy lean clay (CL), sti brown with iron staining	ff, wet, light grayish		 25	U 7				10.7	125.9			
 _ 645				 									
	Sandy lean clay (CL), stil with iron staining	ff, wet, light brown		30	SS 8		4-7-7 N=14		17.5				
_ 640													
 	Sandy lean clay (CL), sti mottled with gray and iro	ff, wet, light brown n staining		35	U 9				22.3	112.9			
 635				 									
	Sandy lean clay with gra dark gray	vel (CL), stiff, wet, 40.0 °		40	U 10				22.9	104.4			
	CONTINUED	NEXT PAGE											
WAT	ER LEVEL OBSERVATIONS						STARTED)	1/23	3/16 FII	NISHE	D 1/23/16	
WD	∑ 6.0 ft	OLSSON ASS 8720 SOUTH 114TH S				107	DRILL CC).		OA DF	RILL RI	G CME 75	
IAD	▼ Not Performed	LA VISTA, I			JUIL	.07	DRILLER			JY LC	GGED	BY SS	

METHOD

HOLLOW STEM AUGER

 Ψ 4.0 ft after 14 Hrs

AD



Sheet 3 of 3

	ASSOCIATES										Sheet 3 of 3		
PROJ	JECT NAME		CLIE								<u> </u>		
	Lincolnway Industrial Rail and Air Par	·k		Clinton Regional Developm						nent Corporation			
PROJ	JECT NUMBER		LOC	ATION									
	015-2757	, ,	Clinton County, Iowa										
NO	Shelby Tube Split Spoon	ပ		YPE R	NOIT	ш	ا مذ	퓠	Υ				
ELEVATION (ft)	MATERIAL DESCRIPTION	GRAPHIC LOG	(#) 40	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS		
	GLACIAL DRIFT												
630			 										
	Sandy lean clay with gravel (CL), very stiff, wet, dark gray		45	U 11				27.2	99.3				
 625													
 	Della de Matas Calif Conson financial i		-										
	Driller's Note: Split Spoon tipped into limestone bedrock	50.0'	50	SS 12		50 /2"		8.3					

BASE OF BORING AT 50.0 FEET

WAT	WATER LEVEL OBSERVATIONS											
WD	☑ 6.0 ft											
IAD	▼ Not Performed											
AD												

STARTED	1/23/16	FINISHED	1/23/16
DRILL CO.	OA	DRILL RIG	CME 75
DRILLER	JY	LOGGED BY	SS
METHOD	HOLLOW S	TEM AUGER	

	OLSSON ®	LOG OF E	BORE	HOL	E NC). В	-6				S	Sheet 1 of 2
PRO	JECT NAME Lincolnway Industr	ial Rail and Air Park		CLI		ton R	egional [Deve	lopm	ent C	orpora	ation
PRO	JECT NUMBER	2757		LOC	ATION		Clinton		-		•	
ELEVATION (ft)	Shelby Tube	Split Spoon	GRAPHIC LOG	DEPTH (ft)	E TYPE IBER	CLASSIFICATION (USCS)			MOISTURE (%)	DRY DENSITY Cot (pcf)	(%)	ADDITIONAL DATA/
ELEV,	MATERIAL DI	ESCRIPTION	GRA	DEI	SAMPLE TYF NUMBER	LASSIF (US	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	SIOM (%)	ORY DI (p	 	REMARKS
		ACE ELEV. (ft): 673	///	0		0						
	AGRICULTURAL ZONE LOESS	6	0.5'									
	Fat clay (CH), stiff, very brown	moist, dark grayish			U 1				26.8	92.6		
_670	GLACIAL DRIFT	3	3.0'	_								
	Clayey sand with gravel moist, reddish brown wit	(SC), loose, slightly h iron staining		 5	SS 2		4-4-2 N=6		13.3			
					1							
	. ☑ Clayey sand with gravel wet, light yellowish brow	(SC), medium dense,			U 3			0.6	13.0	123.1		
665												
	Clayey sand with gravel wet, light brown ▼	(SC), medium dense,		10	U 4				13.8	127.5		
660	Driller's Note Stone Line feet	e encountered at 13			. 1							
	Clayey sand with gravel light brown with iron stain	ning	5.0'	15	SS 5		4-5-8 N=13		13.2			
 655				_								
	Sandy lean clay with gra wet, light brown with iron	staining 2	<u>0.0'</u>	20	SS 6		7-11-14 N=25		11.2			
	CONTINUED	NEXT PAGE										
WAT	ER LEVEL OBSERVATIONS				_		STARTED)	1/24	/16 FII	NISHED) 1/24/16
WD		OLSSON A 8720 SOUTH 114TH				107	DRILL CO			OA DF	RILL RIC	G CME 75
145	▼ 10.5 ft	0.2000011111711	. 51172	, \	-5.1L					1		

LA VISTA, NE 68128

▼ 10.5 ft

Not Performed

IAD

AD

DRILLER

METHOD

CONTINUOUS FLIGHT AUGER

JY LOGGED BY

SS

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	Α	S	S	0	С	L	Α	Т	Ε	S	

LOG OF BOREHOLE NO. B-6

Sheet 2 of 2

	ASSOCIATES									;	Sneet 2 of 2
PRO	JECT NAME		CLIE	NT							
	Lincolnway Industrial Rail and Air Par	k		Clint	ton R	egional I	Deve	lopm	ent C	orpor	ation
PRO	JECT NUMBER		LOC	ATION							
015-2757 Clinton County, Iowa											
NO	Shelby Tube Split Spoon	20	_	YPE R	ATION S)	/6" JE	œ	RE	SITY		
ELEVATION (ft)	MATERIAL DESCRIPTION	GRAPHIC LOG	(#) 20	SAMPLE TYPE NUMBER	CLASSIFICA (USCS)	BLOWS/ N-VALU RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS
 650	GLACIAL DRIFT Sandy lean clay with gravel (CL), hard, wet, light brown with iron staining	25.0'		SS 7		16-25-50 N=75		11.2			

BASE OF BORING AT 25.0 FEET

WAT	WATER LEVEL OBSERVATIONS											
WD	∑ 6.0 ft											
IAD	▼ 10.5 ft											
AD	▼ Not Performed											

STARTED	1/24/16	FINISHED	1/24/16
DRILL CO.	OA	DRILL RIG	CME 75
DRILLER	JY	LOGGED BY	SS
METHOD	CONTINUO	US FLIGHT AU	GER

	OLSSON ®	LOG OF	- BC	RE	HOI	LE NC). B	5-7				5	Sheet 1 of 2
PRO	JECT NAME	ial Bail and Air Bark	,		CLI	ENT	ton D	ogional	Dovo	lonm	ont C	0 K 10 O K	otion
PRO	Lincolnway Industr	iai Raii and Air Park			1.00	CATION	ton R	egional	Deve	iopm	ent C	orpora	ation
11.0	015-	2757				3, 111011		Clinto	1 Col	unty,	Iowa		
ELEVATION (ft)	Shelby Tube MATERIAL DI	Split Spoon ESCRIPTION		GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS
	APPROX. SURFA	ACE ELEV. (ft): 646			0	0)	ರ				-		
	AGRICULTURAL ZONE		0.5'										
645	LOESS												
	Fat clay (CH), stiff, very i brown	moist, dark grayish				U 1				29.4	89.2		
-													
	Lean clay (CL), soft, moi brown with iron staining	st, light yellowish	<i>5</i> 0'			SS 2		1-2-2 N=4		22.9			
	GLACIAL DRIFT		5.0'		5	/ V						_	
640	Clayey sand (SC), medir	n dense, wet, light				U			1.1	16.7	118.5		
	yellowish brown with iron	staining				3			1.1	10.7	110.5		
	Clayey sand (SC), mediu brown	ım dense, wet, light			 10	U 4				15.2	121.3		
635													
	<u> </u>		13.0										
	Sandy lean clay with gra wet, dark grayish brown	vel (CL), very stiff,				U 5				13.0			
					15							_	
630													
	Sandy lean clay with gra wet, grayish brown	vel (CL), very stiff,	20.0			U 6				12.7	126.1		
	CONTINUED	NEXT PAGE	20.0	<i>Y////</i>	20								
WAT	TER LEVEL OBSERVATIONS							STARTE)	1/24	4/16 FII	VISHE	D 1/24/16
WD	∑ 5.0 ft	OLSSO						DRILL CO			OA DF		
IAD	▼ 20.7 ft	8720 SOUTH 114					107	DRILLER					
AD								DRILLER JY LOGGED BY SO METHOD CONTINUOUS FLIGHT AUGER					

METHOD

CONTINUOUS FLIGHT AUGER

	OLSSON ®	OG OF BO	RE	HOL	E NC). B	-7					Sheet 2 of 2
PRO	JECT NAME Lincolnway Industrial Rail and	d Air Park		CLIENT Clinton Regional Development Corporation						ation		
PRO	JECT NUMBER 015-2757			LOCATION Clinton County, lowa								
ELEVATION (ft)	Shelby Tube Split MATERIAL DESCRIPTION	O (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS		
625	Sandy lean clay with gravel (CL), hard dark brown	d, wet, 25.0'			SS 7		11-16-21 N=37		13.8			

WATER LEVEL OBSERVATIONS									
WD	∑ 5.0 ft								
IAD	▼ 20.7 ft								
AD	▼ Not Performed								

STARTED	1/24/16	FINISHED	1/24/16					
DRILL CO.	OA	DRILL RIG	CME 75					
DRILLER	JY	LOGGED BY	SS					
METHOD CONTINUOUS FLIGHT AUGER								

	OLSSON ®	LOG O	F BO	REI	HOL	E NC). В	-8				ç	Sheet 1 of 2
PRO	JECT NAME Lincolnway Industr	ial Rail and Air Par	k		CLI		ton R	egional I	Deve	lopm	ent C		
PRO	JECT NUMBER	2757			LOC	CATION		Clintor				o. por	
ELEVATION (ft)	Shelby Tube MATERIAL D	Split Spoon		GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)		DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS
	APPROX. SURFA AGRICULTURAL ZONE	ACE ELEV. (ft): 664	0.5'		0		0						
	LOESS Fat clay (CH), stiff, very brown					U 1				25.9	88.8		
	GLACIAL DRIFT		3.0'										
_ 660 	Clayey sand (SC), loose brown with iron staining	, slightly moist, light			5	U 2				6.9			
	Clayey sand (SC), loose brown with iron staining	, slightly moist, light				SS 3		4-4-3 N=7		11.9			
655 	Clayey sand (SC), loose brown with iron staining	, slightly moist, light			10	SS 4		3-3-4 N=7		21.5			P-200 = 49.9%
 _ 650 _ 	Driller's Note: Auger ch Line between 13 to 25 fe Poorly graded sand (SP) brown	et 	<u>13.5'</u>		15	SS 5		9-15-23 N=38		18.6			
	. ☑ Poorly graded sand (SP)	, dense, wet, orangish			 	√ ss		9-16-27		18.9			
	brown	NEVT BAGE	20.0'		20	6		N=43					
\A/A=		NEXT PAGE											
WD	FER LEVEL OBSERVATIONS $\ \ \ \ \ \ \ \ \ \ \ \ \ $	OLSSO					40-	STARTED DRILL CO		1/24		NISHEE RILL RI	
		8720 SOUTH 11	4 i H S	IRE	∟⊺ , \$	SUITE '	107						

LA VISTA, NE 68128

DRILLER

METHOD

JY LOGGED BY

CONTINUOUS FLIGHT AUGER

SS

▼ 14.9 ft

Not Performed

IAD

AD

OLSSON ®	
-	۰

LOG OF BOREHOLE NO. B-8

Sheet 2 of

	ASSOCIATES						Sheet 2 of 2				
PRO	JECT NAME Lincolnway Industrial Rail and Air Park		CLIENT Clinton Regional Development Corporation					ation			
PRO	JECT NUMBER 015-2757		LOCATION Clinton County, Iowa								
TION	Shelby Tube Split Spoon	HIC	E	TYPE	SATION S)	S/6" .UE	C. STR. (tsf)	URE	DENSITY (pcf)	<u>.</u>	ADDITIONAL
ELEVATION (ft)	MATERIAL DESCRIPTION	GRAPHIC LOG	20 DEPTH	SAMPLE TYF NUMBER	CLASSIFIC (USCS	BLOWS N-VALU RQD	UNC. S (tsf)	MOISTURE (%)	DRY DEN (pcf	(%)	DATA/ REMARKS
 	GLACIAL DRIFT Driller's Note: Split Spoon tipped into limestone bedrock										
	DAOF OF DODING AT 05 4 FFFT	25.0'	25	SS 7		50 /0.5"					

BASE OF BORING AT 25.0 FEET

WATER LEVEL OBSERVATIONS									
WD	∑ 18.0 ft								
IAD	▼ 14.9 ft								
AD	▼ Not Performed								

STARTED	1/24/16	FINISHED	1/24/16					
DRILL CO.	OA	DRILL RIG	CME 75					
DRILLER	JY	LOGGED BY	SS					
METHOD CONTINUOUS FLIGHT AUGER								

	OLSSON ®	LOG OF BO	RE	HOI	LE NO). B	-9		Sheet 1 of 3			
PRO	JECT NAME Lincolnway Industri	al Rail and Air Park		CLI	ENT Clin	ton R	egional [Deve	lopm	ent C	orpora	ation
PRO	JECT NUMBER 015-2	2757		LO	CATION		Clinton	ı Coı	ıntv.	lowa		
ELEVATION (ft)	Shelby Tube MATERIAL DE	Split Spoon	GRAPHIC LOG	DEPTH (#)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)		DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS
	AGRICULTURAL ZONE			0								
645 	LOESS Fat clay (CH), stiff, slightly brown with organics	1.0' ly moist, dark grayish		 	U 1				14.5	105.5		
	Lean clay (CL), soft, mois iron staining	st, light brown with		 5	SS 2		0-1-2 N=3		18.8			
640		6.0'			,							
_ 040	GLACIAL DRIFT Clayey sand with gravel (wet, light brown with iron	(SC), medium dense,			U 3				16.6	113.8		
		8.0										
	Lean clay (CL), stiff, wet,	light gray		 10	U 4			1.0	21.8	106.7		
635 				 								
 	Lean clay (CL), stiff, wet,	dark gray		 15	U 5	CL			22.1	110.6	29/12	
630												
	Lean clay (CL), stiff, wet,	20.0		20	U 6				29.5	100.3		
WAT	ER LEVEL OBSERVATIONS	NEXT PAGE					CTADTES		4/00)/16 FI	MCUE	1/00/10
WD	✓ 8.0 ft	OLSSON AS 8720 SOUTH 114TH S										

LA VISTA, NE 68128

DRILLER

METHOD

JY LOGGED BY

HOLLOW STEM AUGER

SS

▼ 6.5 ft

Not Performed

IAD

AD

	OLSSON ®	LOG OF	BORE	HOI	E NC). B	-9				Ş	Sheet 2 of 3
PRO	JECT NAME	al Dail and Air David	_	CLII	ENT	40 m D	anianal [2010		ant C		-4: o.u
PRO	Lincolnway Industri	ai Raii and Air Park	<u> </u>	LOC	CATION	ton R	egional [Jeve	opm	ent C	orpor	ation
	015-2	2757					Clinton	Cou	ınty,	lowa		
ELEVATION (ft)	Shelby Tube MATERIAL DE	Split Spoon	GRAPHIC LOG	DEPTH (ft)	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	LL/PI (%)	ADDITIONAL DATA/ REMARKS
	CLACIAL DRIET		////	20		0						
<u>625</u> 	GLACIAL DRIFT			 								
	Lean clay (CL), very stiff,	wet, dark gray		25	U 7				22.8	109.3		
<u>620</u> 												
 615	Lean clay (CL), stiff, wet,	gray		30	U 8				20.2			
 	Poorly graded sand with s dense, wet, light grayish b	silt (SP-SM), medium prown	33.0'	35	SS 9		2-4-6 N=10					
	Poorly graded sand with sidense, wet, light grayish b	brown	40.0'	40	SS 10		4-7-8 N=15		14.5			P-200 = 6.3%
WAT	TER LEVEL OBSERVATIONS						STARTED	1	1/03	N/16 EII	NISHEI) 1/23/16
WD	∑ 8.0 ft		N ASSOCI				DRILL CO				RILL RI	
IAD	▼ 6.5 ft	8720 SOUTH 114	4TH STRE STA, NE 6			107	DRILLER	•			GGED	
AD	▼ Not Performed		J 1.77, 14L 00	J 120			METHOD	<u></u> Ц			MALIC	



	ASSOCIATES	L00 01	DOIL			<i>.</i>					5	Sheet 3 of 3
PRO	JECT NAME			CLI	ENT							
	Lincolnway Industrial R	Rail and Air Park			Clin	ton R	egional l	Deve	lopm	ent C	orpor	ation
PRO	JECT NUMBER			LOCATION								
ļ.,,	015-2757	7					Clintor	1 Col	ınty,	lowa		
N O	Shelby Tube	Split Spoon	<u>១</u>	_	YPE :R	ATION (.e. IE	مخ	E E	SITY		
ELEVATION (ft)	MATERIAL DESCR	RIPTION	GRAPHIC	(#) OEPTH	SAMPLE TYPE NUMBER	CLASSIFICATION (USCS)	BLOWS/6" N-VALUE RQD	UNC. STR. (tsf)	MOISTURE (%)	DRY DENSITY (pcf)	(%)	ADDITIONAL DATA/ REMARKS
	GLACIAL DRIFT											
_ 605 _	Poorly graded sand with silt (\$ dense, wet, light grayish brow	SP-SM), medium n		 								
			43.5'	-								
	Clayey sand with gravel (SC), wet, yellowish brown with iron			 _ 45	SS 11		4-5-6 N=11					
600												
	Clayey sand with gravel (SC), wet, yellowish brown with iron	staining	50.0'	50	SS 12		5-6-8 N=14		13.7			
1	BASE OF BORING AT	50.0 FEET										

BASE OF BORING AT 50.0 FEET

WAT	WATER LEVEL OBSERVATIONS								
WD	∑ 8.0 ft								
IAD	▼ 6.5 ft								
AD	▼ Not Performed								

STARTED	1/23/16	FINISHED	1/23/16				
DRILL CO.	OA	DRILL RIG	CME 75				
DRILLER	JY	LOGGED BY	SS				
METHOD HOLLOW STEM AUGER							



SUMMARY OF LABORATORY RESULTS PAGE 1 OF 3

PROJECT NAME Lincolnway Industrial Rail and Air Park

CLIENT Clinton Regional Development Corporation

PROJECT NUR	PROJECT NUMBER 015-2757	57					4	ROJECT LOC	PROJECT LOCATION Clinton County, lowa	n County, Iowa		
BORING	SAMPLE		MOISTURE	DRY	VOID	SATURATION			ATTERBERG LIMITS	ITS	6	nscs
NUMBER	l'D.	DEPTH (ft)	CONTENT (%)	DENSITY (pcf)	RATIO	(%)	SIKENGIH (tsf)	LIQUID	PLASTIC LIMIT	PLASTICITY INDEX	P-200	CLASS.
B-1	N-1	1.0 - 2.5'	22.9	8.66	0.688	89.6	1.1					
B-1	N-2	3.5 - 5.0'	17.3									
B-1	SS-3	6.0 - 7.5'	17.2									
B-1	SS-4	8.5 - 10.0'	18.9								25.9	
B-1	SS-5	13.5 - 15.0'										
B-1	9-SS	18.5 - 20.0'	18.8								3.5	
B-1	SS-7	23.5 - 25.0'										
B-2	N-1	1.0 - 2.5'	26.3	95.4	0.767	92.5		29	48	19		СН
B-2	SS-2	3.5 - 5.0'	14.8									
B-2	SS-3	6.0 - 7.5'	14.6									
B-2	SS-4	8.5 - 10.0'	15.1								34.3	
B-2	NR-5	13.5 - 15.0'										
B-2	SS-5	15.0 - 16.5'	13.6									
B-2	9-SS	18.5 - 20.0'	17.8									
B-2	1-0	23.5 - 25.0'	13.5	105.2	0.603	60.3						
B-3	N-1	1.0 - 2.5'	22.9	105.1	0.604	100.0						
B-3	N-2	3.5 - 5.0'	12.8									
В-3	SS-3	6.0 - 7.5'	20.6									
B-3	SS-4	8.5 - 10.0'	19.3								9.6	
В-3	SS-5	13.5 - 15.0'										
B-3	SS-6	18.5 - 20.0'										
B-3	SS-7	23.5 - 25.0'	36.5					41	22	16		CL
B-3	SS-8	28.5 - 30.0'	39.7									
B-3	6-N	30.0 - 31.5'	38.2	88.6	0.902	100.0						
B-3	N-10	33.5 - 35.0'	38.7									
B-3	U-11	38.5 - 40.0'	21.6	98.0	0.719	81.1						
В-3	N-12	43.5 - 45.0'	17.8									
B-3	N-13	48.5 - 50.0'	13.7	127.4	0.323	100.0						
B-3	SS-13	50.0 - 51.5'	14.0									
B 4	n-1	1.0 - 2.5'	28.4	91.2	0.849	90.3						



SUMMARY OF LABORATORY RESULTS PAGE 2 OF 3

PROJECT NAME Lincolnway Industrial Rail and Air Park

CLIENT Clinton Regional Development Corporation

92.6 123.1 127.5 89.2	8.3 26.8 13.3 13.0 13.2 11.2 11.2 11.2
0.423 100.0	118.5 0.423
	92.0



SUMMARY OF LABORATORY RESULTS

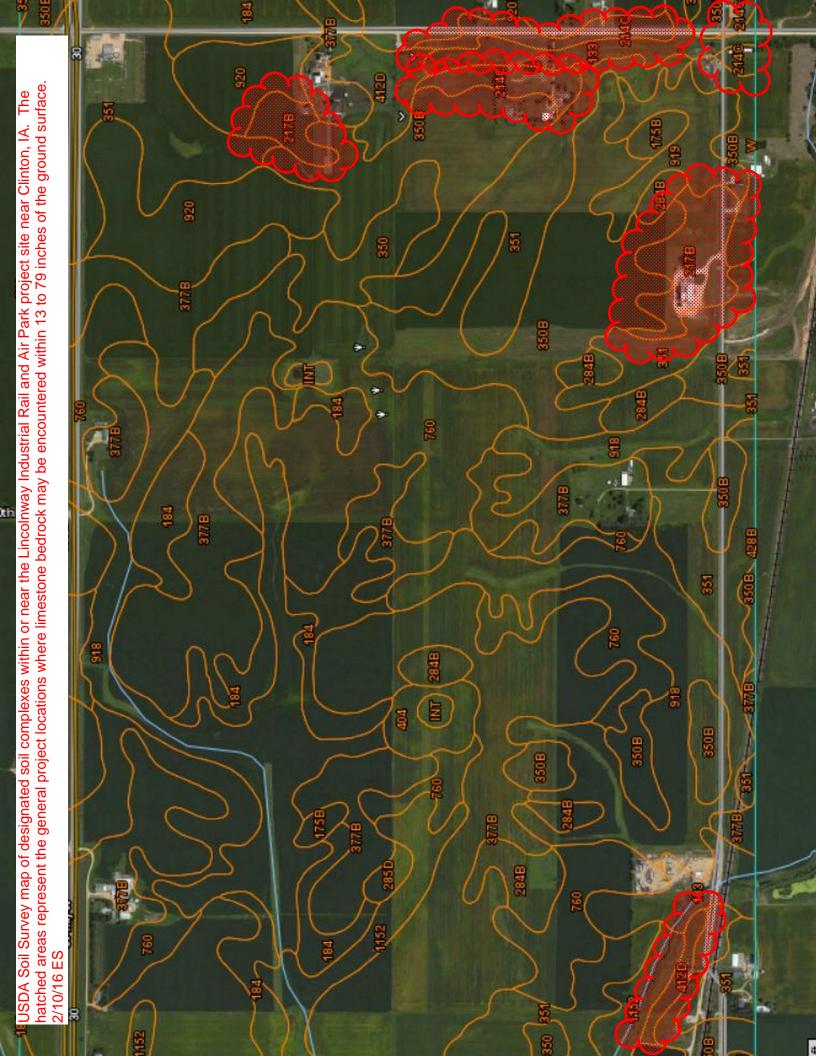
PAGE 3 OF 3

PROJECT NAME Lincolnway Industrial Rail and Air Park

CLIENT Clinton Regional Development Corporation

PROJECT LOCATION Clinton County, lowa

USCS CLASS. 占 P-200 0 6.3 49 PLASTICITY INDEX 7 ATTERBERG LIMITS PLASTIC LIMIT 7 LIQUID 29 SATURATION STRENGTH (%) (tsf) 0. 100.0 100.0 100.0 100.0 100.0 93.0 77.8 65.5 0.899 0.579 0.524 0.680 0.542 0.598 0.481 VOID RATIO 0.337 MOISTURE DRY CONTENT (%) DENSITY (pcf) 105.5 113.8 110.6 100.3 109.3 126.1 88.8 106.7 13.8 25.9 11.9 21.5 18.9 14.5 18.8 16.6 21.8 29.5 22.8 20.2 14.5 18.6 22.1 12.7 6.9 13.7 48.5 - 50.0' 18.5 - 20.0' 23.5 - 25.0' 18.5 - 20.0' 23.5 - 25.0' 28.5 - 30.0' 33.5 - 35.0' 38.5 - 40.0' 43.5 - 45.0' 13.5 - 15.0' 18.5 - 20.0' 24.0 - 25.0' 13.5 - 15.0' 8.5 - 10.0' SAMPLE DEPTH (ft) 8.5 - 10.0' 1.0 - 2.53.5 - 5.0' 6.0 - 7.5 1.0 - 2.5 6.0 - 7.53.5 - 5.0' PROJECT NUMBER 015-2757 SAMPLE I.D. SS-12 **SS-10 SS-11 SS-5 88-9 SS-3 SS-6 SS-7 SS-2 28-7 SS-4** U-5 **φ φ** ⊃ φ | **U-**2 5 ლ<u>-</u>ე 4 5 **U-7** BORING NUMBER ф М φ Μ ф В 8 Β ф В ф М <u>ф</u> 6-B 6-6-6-B 6-9 6-B 6-B-9 6-8 6-B <u>မ</u> <u>ရ</u> 6-9-6-B **B-7** B-7



Engineering Properties

This table gives the engineering classifications and the range of engineering properties for the layers of each soil in the survey area.

Hydrologic soil group is a group of soils having similar runoff potential under similar storm and cover conditions. The criteria for determining Hydrologic soil group is found in the National Engineering Handbook, Chapter 7 issued May 2007(http:// directives.sc.egov.usda.gov/OpenNonWebContent.aspx?content=17757.wba). Listing HSGs by soil map unit component and not by soil series is a new concept for the engineers. Past engineering references contained lists of HSGs by soil series. Soil series are continually being defined and redefined, and the list of soil series names changes so frequently as to make the task of maintaining a single national list virtually impossible. Therefore, the criteria is now used to calculate the HSG using the component soil properties and no such national series lists will be maintained. All such references are obsolete and their use should be discontinued. Soil properties that influence runoff potential are those that influence the minimum rate of infiltration for a bare soil after prolonged wetting and when not frozen. These properties are depth to a seasonal high water table, saturated hydraulic conductivity after prolonged wetting, and depth to a layer with a very slow water transmission rate. Changes in soil properties caused by land management or climate changes also cause the hydrologic soil group to change. The influence of ground cover is treated independently. There are four hydrologic soil groups, A, B, C, and D, and three dual groups, A/D, B/D, and C/D. In the dual groups, the first letter is for drained areas and the second letter is for undrained areas.

The four hydrologic soil groups are described in the following paragraphs:

Group A. Soils having a high infiltration rate (low runoff potential) when thoroughly wet. These consist mainly of deep, well drained to excessively drained sands or gravelly sands. These soils have a high rate of water transmission.

Group B. Soils having a moderate infiltration rate when thoroughly wet. These consist chiefly of moderately deep or deep, moderately well drained or well drained soils that have moderately fine texture to moderately coarse texture. These soils have a moderate rate of water transmission.

Group C. Soils having a slow infiltration rate when thoroughly wet. These consist chiefly of soils having a layer that impedes the downward movement of water or soils of moderately fine texture or fine texture. These soils have a slow rate of water transmission.

Group D. Soils having a very slow infiltration rate (high runoff potential) when thoroughly wet. These consist chiefly of clays that have a high shrink-swell potential, soils that have a high water table, soils that have a claypan or clay layer at or near the surface, and soils that are shallow over nearly impervious material. These soils have a very slow rate of water transmission.

Depth to the upper and lower boundaries of each layer is indicated.

Texture is given in the standard terms used by the U.S. Department of Agriculture. These terms are defined according to percentages of sand, silt, and clay in the fraction of the soil that is less than 2 millimeters in diameter. "Loam," for example, is soil that is 7 to 27 percent clay, 28 to 50 percent silt, and less than 52 percent sand. If the content of particles coarser than sand is 15 percent or more, an appropriate modifier is added, for example, "gravelly."

Classification of the soils is determined according to the Unified soil classification system (ASTM, 2005) and the system adopted by the American Association of State Highway and Transportation Officials (AASHTO, 2004).

The Unified system classifies soils according to properties that affect their use as construction material. Soils are classified according to particle-size distribution of the fraction less than 3 inches in diameter and according to plasticity index, liquid limit, and organic matter content. Sandy and gravelly soils are identified as GW, GP, GM, GC, SW, SP, SM, and SC; silty and clayey soils as ML, CL, OL, MH, CH, and OH; and highly organic soils as PT. Soils exhibiting engineering properties of two groups can have a dual classification, for example, CL-ML.

The AASHTO system classifies soils according to those properties that affect roadway construction and maintenance. In this system, the fraction of a mineral soil that is less than 3 inches in diameter is classified in one of seven groups from A-1 through A-7 on the basis of particle-size distribution, liquid limit, and plasticity index. Soils in group A-1 are coarse grained and low in content of fines (silt and clay). At the other extreme, soils in group A-7 are fine grained. Highly organic soils are classified in group A-8 on the basis of visual inspection.

If laboratory data are available, the A-1, A-2, and A-7 groups are further classified as A-1-a, A-1-b, A-2-4, A-2-5, A-2-6, A-2-7, A-7-5, or A-7-6. As an additional refinement, the suitability of a soil as subgrade material can be indicated by a group index number. Group index numbers range from 0 for the best subgrade material to 20 or higher for the poorest.

Rock fragments larger than 10 inches in diameter and 3 to 10 inches in diameter are indicated as a percentage of the total soil on a dry-weight basis. The percentages are estimates determined mainly by converting volume percentage in the field to weight percentage.

Percentage (of soil particles) passing designated sieves is the percentage of the soil fraction less than 3 inches in diameter based on an ovendry weight. The sieves, numbers 4, 10, 40, and 200 (USA Standard Series), have openings of 4.76, 2.00, 0.420, and 0.074 millimeters, respectively. Estimates are based on laboratory tests of soils sampled in the survey area and in nearby areas and on estimates made in the field.

Liquid limit and plasticity index (Atterberg limits) indicate the plasticity characteristics of a soil. The estimates are based on test data from the survey area or from nearby areas and on field examination.

References:

American Association of State Highway and Transportation Officials (AASHTO). 2004. Standard specifications for transportation materials and methods of sampling and testing. 24th edition.

American Society for Testing and Materials (ASTM). 2005. Standard classification of soils for engineering purposes. ASTM Standard D2487-00.

Report—Engineering Properties

possible textures follow the dash. The criteria for determining the hydrologic soil group for individual soil components is found in the National Engineering Handbook, Chapter 7 issued May 2007(http://directives.sc.egov.usda.gov/ Absence of an entry indicates that the data were not estimated. The asterisk ** denotes the representative texture; other OpenNonWebContent.aspx?content=17757.wba).

				Engineeri	Engineering Properties-Clinton County, lowa	es-Clinton	County, Ic	owa						
Map unit symbol and	Pct. of	Hydrolo	Depth	USDA texture	Classif	Classification	Fragn	Fragments	Percenta	Percentage passing sieve number—	g sieve n	nmber—	Liquid	Plasticit
soli name	map unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200	Ĕ	y index
			uI				Pct	Pct					Pct	
133—Colo sifty clay loam, 0 to 2 percent slopes														
Colo, occasionally flooded	92	C/D	0-38	Silty clay loam	CL, CH	A-7	0-0-0	0 -0 -0	100-100 -100 -100		90-95-1 00	90-95-1 00	40-50 -60	15-23-3 0
			38-60	Silty clay loam, silt loam, clay loam	CH, CL	A-7	0-0-0	0-0-0	100-100	100-100	95-98-1 00	80-90-1 00	40-48 -55	15-23-3 0
175B—Dickinson fine sandy loam, 2 to 5 percent slopes														
Dickinson	92	ď	8-0	Loam, sandy loam, fine sandy loam	SC-SM, SC	A-6, A-4	0-0-0	0-0-0	100-100	100-100	96-93- 96	41-44- 47	25-29 -33	7-9 -11
			8-18	Loam, sandy loam, fine sandy loam	SC, SC- SM	A-6, A-4	0-0-0	0- 0- 0	100-100	100-100	89-93- 97	39-43- 47	21-27 -33	6-9 -11
			18-30	Sandy Ioam, fine sandy Ioam	SC, SC- SM	A-6, A-4	0-0-0	0-0-0	100-100 -100 -100	100-100	89-93- 97	39-43- 47	20-24 -28	6-9 -12
			30-36	Loamy sand, sand, fine sand, loamy fine sand	SC-SM, SM	A-2-4	0-0-0	0-0-0	100-100	100-100	78-80- 83	21-23- 26	16-18 -21	2-3 -6
			36-60	Fine sand, loamy fine sand, loamy sand	SM, SP- SM, SC- SM	A-2-4	0-0-0	0-0-0	100-100 100-100 -100 -100		78-80- 83	12-14-	16-18	2-3 -6

				Engineer	Engineering Properties-Clinton County, lowa	es-Clinton	County, Ic	wa						
Map unit symbol and	Pct. of	Hydrolo	Depth	USDA texture	Classif	Classification	Fragr	Fragments	Percenta	ge passin	Percentage passing sieve number—	umber—	Liquid	Plasticit
son name	unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200	Ĭ	y Index
			4				Pct	Pct					Pct	
177B—Saude loam, 2 to 5 percent slopes														
Saude	06	В	2-0	Sandy loam, loam	SC, ML	A-6, A-4, A-7-6	0-0-0	0-0-0	100-100	83-98-1	67-83- 90	45-58- 64	32-37 -43	9-12-16
			7-13	Loam, sandy loam	CL, SC, ML	A-6, A-4, A-7-6	0-0-0	0-0-0	100-100	83-98-1	67-83- 90	45-58- 64	30-36 -43	9-12-16
			13-16	Loam, sandy loam	SC, CL	A-6, A-4	0-0-0	0- 0- 1	88-98-1	85-96-1 00	70-82- 92	46-55- 64	27-32 -40	9-12-16
			16-24	Loam, sandy loam	SC, SC- SM, CL	A-4, A-6	0-0-0	0- 1- 3	95-96-1 00	86-89-1	69-74- 86	43-48- 56	24-27 -30	7-10-12
			24-28	Sandy loam, loam	SC-SM, CL, SC	A-6, A-4	0-0-0	0- 1- 3	95-96-1 00	86-89-1	67-73- 84	40-44- 52	24-27 -30	7-10-12
			28-36	Loamy sand, sand, loamy coarse sand, gravelly coarse sand, coarse sand	SW-SM, SC-SM, SC	A-1-a, A-2-4	0-0-0	0- 1- 6	74-91- 97	41-82- 92	24-55- 69	7-20- 29	0-19 -26	NP-7 -9
			36-79	Loamy sand, gravelly coarse sand, sand, coarse sand, loamy coarse sand	SP-SC, SC, SW-SM	A-1-b, A-1-a, A-2-4	0-0-0	0-5-6	74-88- 97	41-70- 92	24-45- 69	5-12- 24	5-12- 24 0-17 -26 NP-6 -9	NP-6 -9
184—Klinger silt loam, 1 to 3 percent slopes														
Klinger	95	C/D	0-17	Silt loam	CL	A-6	0-0-0	0-0-0	100-100	100-100	100-100	95-98-1 00	30-35 -40	10-15-2 0
			17-31	Silty clay loam	CL	A-7	0 -0 -0	0-0-0	100-100 -100	100-100	100-100	95-98-1 00	40-45 -50	20-25-3 0
			31-60	Loam, clay loam	CL	A-6	0-0-0	0-3-5	90-93- 95	85-88- 90	75-80- 85	55-60- 65	25-30 -35	10-15-2 0

				Engineeri	ing Properti	Engineering Properties-Clinton County, lowa	County, k	owa						
Map unit symbol and Pct. of Hydrolo	Pct. of	Hydrolo	Depth	USDA texture	Classif	Classification	Fragn	Fragments	Percenta	ge passir	Percentage passing sieve number—	number—	Liquid	Plasticit
soil name	unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		y index
			u				Pct	Pct					Pct	
214B—Rockton loam, 20 to 30 inches to limestone, 2 to 5 percent slopes														
Rockton, 20 to 30 inches to bedrock	100 C	O	0-14	Loam	CL, ML, CL-ML	A-4	0-0-0	0-0-0	90-95-1 00	90-95-1	85-90- 95	50-63- 75	25-30 -35	5-8 -10
			14-25	Sandy clay loam, loam, clay loam	SC, CL	A-6, A-7	0-0-0	0-0-0	90-95-1 00	90-95-1	75-83- 90	45-58- 70	30-38 -45	10-15-2 0
			25-38	Bedrock	ı	I	0-0-0	0-0-0	ı	ı	1	ı	1	1
214C—Rockton loam, 20 to 30 inches to limestone, 5 to 9 percent slopes														
Rockton, 20 to 30 inches to bedrock	100 C	O	0-14	Loam	CL-ML, CL, ML	A-4	0-0-0	0-0-0	90-95-1 00	90-95-1 00	85-90- 95	50-63- 75	25-30 -35	5-8 -10
			14-25	Loam, clay loam, sandy clay loam	CL, SC	A-7, A-6	0 -0 -0	0-0-0	90-95-1 00	90-95-1 00	75-83- 90	45-58- 70	30-38 -45	10-15-2 0
			25-38	Bedrock	1		0 -0 -0	0-0-0	1			1	1	1

													1	
	Plasticit	y index			7-15-18	7-16-19	12-18-2 5	12-21-2 5	13-21-2 5	ı		5-8 -10	5-8 -10	NP P
	Liquid		Pct		29-40 -45	27-39 -43	30-39 -49	29-41 -47	30-40 -45	ı		15-20 -25	15-20 -25	0-7 -14
	umber—	200			83-93- 98	81-93- 96	86-94-1 00	83-95-1 00	58-73- 83	ı		25-33- 40	25-33- 40	3-8-12
	Percentage passing sieve number—	40			89-99-1 00	88-100- 100	92-100- 100	87-99-1 00	73-90- 97	ı		60-65- 70	50-60- 70	20-30- 40
	ige passir	10			100-100 -100	100-100 -100	100-100	100-100	81-91- 97	ı		-66-06	90-93- 95	70-72- 85
	Percenta	4			100-100 -100	100-100 -100	100-100 -100	100-100	91-95- 98	ı		95-98-1 00	95-98-1	70-73- 90
owa	Fragments	3-10 inches	Pct		0 -0 -0	0-0-0	0-0-0	0-0-0	0- 1- 4	1		0 -0 -0	0-0-0	0-3-5
County, Io	Fragr	>10 inches	Pct		0 -0 -0	0 -0 -0	0-0-0	0-0-0	0-0-0	1		0 -0 -0	0-0-0	0 -0 -0
es-Clinton	cation	AASHTO			A-6	A-6	A-6	A-6, A-7-6	A-6			A-4, A-2	A-4, A-2	A-1
Engineering Properties-Clinton County, lowa	Classification	Unified			ML, CL- ML, CL	ML, CL- ML, CL	ML, CL- ML, CL	CL	SC, CL	ı		SC-SM, SC	SC-SM, SC	SP, SW- SM, SW, SP-SM
Engineeri	USDA texture				Silt loam	Silt loam	Silt loam	Silt Ioam, silty clay Ioam	Sandy clay loam, clay loam, loam	Bedrock		Sandy loam	Sandy loam	Gravelly sand, loamy sand, sand
	Depth		u		6-0	9-11	11-23	23-32	32-34	34-79		0-19	19-35	35-60
	Hydrolo	group			C							٧		
	Pct. of	unit			100							100		
	Map unit symbol and	SOIL DATE		217B—Ripon silt loam, 30 to 40 inches to limestone, 2 to 5 percent slopes	Ripon, 30 to 40 inches to bedrock						284B—Flagler sandy loam, 1 to 5 percent slopes	Flagler		

			Engineer	Engineering Properties-Clinton County, lowa	ies-Clinton	County, Ic	wa						
4	Hydrolo	Depth	USDA texture	Classif	Classification	Fragments	ents	Percenta	ge passin	Percentage passing sieve number—	umber—	Liquid	Plasticit
map unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200	Ĭ	y Index
		n				Pct	Pct					Pct	
100	⋖	0-17	Sandy loam	SM, SC- SM	A-2, A-4	0-0-0	0-0-0	95-98-1 00	90-95-1 00	55-63- 70	25-33- 40	15-21 -26	2-5 -7
		17-22	Sandy loam, loamy sand	ML, SM, SC, CL	A-2, A-4	0-0-0	0-0-0	95-98-1 00	85-93-1 00	50-73- 95	25-50- 75	15-23 -30	2-6 -10
		22-60	Very gravelly loamy sand, coarse sand, gravelly coarse sand	SP, SP- SM	A-1	0 -0 -0	0-0-0	50-68- 85	45-65- 85	20-28- 35	1-3-5	0-7 -14	ФN
92	В	0-15	Silt loam	ML	A-4	0 -0 -0	0 -0 -0	95-98-1 00	95-98-1 00	95-98-1 00	85-90- 95	25-33 -40	3-7 -10
		15-35	Silt loam, loam	CL-ML, CL	A-4, A-6	0-0-0	0-0-0	95-98-1 00	95-98-1 00	95-98-1 00	85-90- 95	25-33 -40	5-10-15
		35-60	Very gravelly loamy sand, gravelly coarse sand, gravelly sand	SW, SP, SP-SM	A-1	0 -0 -0	0-1-2	80-88- 95	65-75- 85	30-40- 50	3- 7- 10	0-7 -14	N ⊕

		Engineer	Engineering Properties-Clinton County, lowa	ies-Clinton	County, Ic	wa						
Hydrolo Depth		USDA texture	Classif	Classification	Fragments	ents	Percenta	ye passin	Percentage passing sieve number—	umber—	Liquid	Plasticit
group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		у іпаех
u					Pct	Pct					Pct	
0-15 Si	S	Silt loam	ML	A-4	0-0-0	0-0-0	95-98-1 00	95-98-1 00	95-98-1 00	85-90- 95	25-33 -40	3-7 -10
15-35 Lo	2	Loam, silt loam	CL, CL- ML	A-6, A-4	0-0-0	0-0-0	95-98-1 00	95-98-1 00	95-98-1 00	85-90- 95	25-33 -40	5-10-15
35-60 Ver	> 8 8 0	Very gravelly loamy sand, gravelly sand, gravelly coarse sand	SW, SP, SP-SM	A-1	0-0-0	0-1-2	80-88- 95	65-75- 85	30-40- 50	3- 7- 10	0-7 -14	₽
C/D 0-16 Silt lo	Silt	loam	CL, CL- ML	A-6, A-4	0-0-0	0-0-0	100-100 -100	100-100 -100	100-100	95-98-1 00	25-33 -40	5-10-15
16-46 Silty	Silt	y clay loam	CF	A-7	0 -0 -0	0 -0 -0	100-100 -100	100-100 -100	100-100 -100	95-98-1 00	40-45 -50	20-25-3 0
46-50 Sa	Sa	Sandy Ioam	SC, SC- SM	A-4	0-0-0	0-0-0	100-100 -100	95-98-1 00	80-85- 90	35-43- 50	20-25 -30	5-8 -10
50-60 Sa	Sa	Sand, loamy sand	SM	A-3, A-2	0-0-0	0-0-0	100-100 95-98-1 -100 00		80-85- 90	5-13- 20 15-18	15-18 -20	NP-3 -5

				Engineer	ing Propert	Engineering Properties-Clinton County, lowa	County, Ic	owa						
Map unit symbol and Pct. of	Pct. of	I	Depth	USDA texture	Classif	Classification	Fragments	nents	Percenta	Percentage passing sieve number—	g sieve n	nmber—	Liquid	Plasticit
soli name	map unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200	Ĕ	y index
			Ч				Pct	Pct					Pct	
377B—Dinsdale silt loam, 2 to 5 percent slopes														
Dinsdale	06	O	2-0	Silty clay loam, silt loam	ر ا	A-7-6, A-6 0-0-0	0-0-0	0-0-0	100-100	100-100	96-99-1 00	91-94- 99	40-43 -50	16-17-2 2
			7-12	Silt loam, silty clay loam	ರ	A-6, A-7-6	0-0-0	0-0-0	100-100	100-100	92-99-1 00	87-94- 95	38-46 -48	16-22-2 2
			12-19	Silty clay loam	ر ا	A-7-6	0-0-0	0-0-0	100-100	100-100	92-99-1 00	87-94- 95	36-44 -46	16-22-2 2
			19-28	Silty clay loam	ਹ	A-7-6	0-0-0	0-0-0	100-100	100-100	94-98-1	89-93- 97	38-43 -49	19-22-2 5
			28-34	Silty clay loam	J J	A-7-6	0-0-0	0- 1- 3	92-96-1 00	85-92- 98	80-91- 98	75-86- 95	38-43 -49	19-22-2 5
			34-46	Clay loam, loam	ರ	A-6	0-0-0	0- 0- 4	91-98- 98	82-95- 97	71-85- 93	47-57- 65	30-33 -41	13-16-2
			46-58	Clay loam, loam	C	A-6	0-0-0	0-0-2	92-98- 98	85-95- 97	70-85- 88	45-57- 60	33-35 -38	16-17-1 9
			58-80	Clay loam, loam	7	A-6	0-0-0	0-0-2	92-97- 98	85-94- 97	70-84- 88	45-56- 60	33-35 -38	16-17-1

				Engineeri	ing Propert	Engineering Properties-Clinton County, lowa	County, Ic	owa						
Map unit symbol and Po		Hydrolo	Depth	USDA texture	Classif	Classification	Fragn	Fragments	Percenta	Percentage passing sieve number-	ig sieve n	umber—	Liquid	Plasticit
	map unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200	Ĕ E	y index
			Ш				Pct	Pct					Pct	
404—Thorp silt loam, 0 to 2 percent slopes														
	100	B/D	0-14	Silt loam	7	A-6, A-4	0-0-0	0-0-0	95-98-1 00	95-98-1 00	90-95-1 00	75-85- 95	20-30 -40	8-14-19
			14-19	Silt loam	7	A-6, A-4	0-0-0	0-0-0	95-98-1 00	95-98-1 00	90-95-1 00	75-85- 95	25-30 -35	7-11-15
			19-43	Silty clay loam, silt loam	7	A-6, A-7	0-0-0	0-0-0	95-98-1 00	95-98-1 00	90-95-1 00	75-85- 95	35-43 -50	13-20-2 7
			43-50	Silt loam, sandy clay loam, clay loam	C	A-6, A-7, A-4	0-0-0	0-0-0	90-95-1 00	90-95-1 00	90-95-1 00	70-80- 90	20-35 -50	8-17-26
			50-65	Sand, sandy loam	SM, SC- SM, CL- ML, ML	A-4, A-2	0-0-0	0-0-0	85-93-1 00	75-85- 95	65-75- 85	20-40- 60	15-18 -20	NP-3 -6
412D—Sogn loam, 5 to 14 percent slopes														
	100	D	0-11	Clay loam, loam	ر ا	A-6, A-7-6	0-0-0	0- 5- 10	85-93-1 00	85-93-1 00	80-90-1 00	65-73- 80	35-40 -45	15-18-2 0
			11-13	Loam	ر ا	A-6	0 -0 -0	0- 5- 10	85-93-1 00	85-93-1 00	85-93-1 00	70-85-1 00	25-33 -40	11-17-2 3
			13-17	Bedrock	ı	ı	0-0-0	0-0-0	-			1	1	
428B—Ely silt loam, 2 to 5 percent slopes														
	95	C/D	0-26	Silt loam	CL	A-6, A-7	0 -0 -0	0-0-0	100-100	100-100	95-98-1 00	95-98-1 00	30-38 -45	10-18-2 5
			26-60	Silty clay loam	CL, ML	A-6, A-7	0 -0 -0	0 -0 -0	100-100	100-100	95-98-1 00	95-98-1 00	35-43 -50	10-18-2 5
			60-64	Silt Ioam, Ioam, silty clay Ioam	CF	A-6	0-0-0	0-0-0	100-100	100-100	90-95-1 00	85-93-1 00	25-33 -40	10-15-2 0

				Engineer	Engineering Properties-Clinton County, lowa	ies-Clinton	County, Ic	wa						
l and	Pct. of	Hydrolo	Depth	USDA texture	Classif	Classification	Fragments	nents	Percenta	ge passir	Percentage passing sieve number—	umber—	Liquid	Plasticit
000 0000 0000 0000 0000 0000 0000 0000 0000	unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		y muex
			u				Pct	Pct					Pct	
499F—Nordness silt loam, 14 to 25 percent slopes														
Nordness	100	Q	0-12	Silt loam	CL-ML, CL	A-4	0-0-0	0-0-0	100-100	100-100	90-95-1 00	70-80- 90	20-25 -30	5-8 -10
			12-15	Silt Ioam, Ioam, silty clay Ioam	CL	A-7, A-6	0 -0 -0	0-0-0	100-100	100-100	90-95-1 00	70-80- 90	30-38 -45	15-20-2 5
			15-19	Clay loam, silty clay loam, loam, loam	CL	A-6, A-7	0 -0 -0	2- 6- 10	85-90- 95	90-85-	70-78- 85	65-75- 85	30-38 -45	15-20-2 5
			19-60	Weathered bedrock, bedrock	ı	1	0-0-0	0-0-0					1	ı
760—Ansgar silt loam, 0 to 3 percent slopes														
Ansgar	92	C/D	8-0	Silt loam	CL, CL- ML	A-4, A-6	0-0-0	0-0-0	100-100	100-100	95-99-1 00	89-93- 96	34-39 -43	11-15-1
			8-12	Silt loam	CL-ML, CL	A-4, A-6	0 -0 -0	0-0-0	100-100	100-100	95-99-1 00	89-93- 96	28-33 -37	12-15-1 7
			12-18	Silty clay loam	CL	A-7-6	0 -0 -0	0-0-0	100-100	100-100	97-99-1 00	93-95- 97	40-42 -45	21-23-2 4
			18-28	Silty clay loam	CL	A-7-6	0 -0 -0	0-0-0	100-100	100-100	97-99-1 00	93-95- 97	40-42 -45	21-23-2 4
			28-36	Clay loam, loam	CL	A-6	0 -0 -0	2- 3- 15	91-92- 98	82-88- 98	78-87- 98	63-71- 83	29-34 -39	13-16-1 9
			36-68	Clay loam, loam	CL	A-6	0 -0 -0	2- 3- 15	91-92- 98	82-88- 98	78-87- 98	63-71- 83	29-34 -39	13-16-1 9
			68-80	Clay loam, loam	ر ا	A-6	0-0-0	2- 3- 15	91-92- 98	82-88- 98	77-87- 98	56-65- 76	29-35 -39	12-17-1 9

				Engineeri	Engineering Properties-Clinton County, lowa	ies-Clinton	County, Ic	owa						
۱ م	<u>_</u>	Hydrolo	Depth	USDA texture	Classif	Classification	Fragn	Fragments	Percenta	ge passir	Percentage passing sieve number—	umber-	Liquid	Plasticit
	unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		y index
			u				Pct	Pct					Pct	
	95	95 C/D	8-0	Silty clay loam	ML, MH	A-7-5	0-0-0	0-0-0	100-100	100-100	97-100- 100	95-99-1 00	49-54 -58	18-21-2 4
			8-12	Silty clay loam	CL, MH	A-7-5, A-7-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	95-99-1 00	44-52 -56	19-22-2 4
			12-18	Silty clay loam	CH, CL	A-7-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	95-99-1 00	41-49 -53	19-23-2 5
			18-22	Silty clay loam	CH, CL	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	95-99-1 00	40-48 -50	19-24-2 5
			22-27	Silty clay loam	CF	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	95-99-1 00	39-46 -49	19-23-2 5
			27-36	Silty clay loam	CL	A-6, A-7-6	0 -0 -0	0-0-0	100-100	100-100 -100	97-100- 100	95-99-1 00	39-45 -49	19-23-2 5
			36-42	Silty clay loam	CF	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	95-99-1 00	38-43 -48	19-22-2 5
			42-47	Silty clay loam, silt loam	CF	A-6, A-7-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	94-98-1 00	34-37 -43	16-18-2 3
			47-50	Silt Ioam, silty clay Ioam	CL	A-6, A-7-6 0- 0- 0	0-0-0	0-0-0	100-100	100-100	97-100- 100	94-98-1 00	34-35 -43	16-17-2 3
			50-79	Loamy fine sand	SC-SM, SM	A-2-4	0-0-0	0-0-0	100-100	100-100	89-95-1 00	18-23- 30	16-19 -22	2-4 -6

				Engineer	ing Properti	Engineering Properties-Clinton County, lowa	County, k	owa						
land	Pct. of	Pct. of Hydrolo	Depth	USDA texture	Classif	Classification	Fragments	nents	Percenta	Percentage passing sieve number—	g sieve n	nmber-	Liquid	Plasticit
soll name	unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		y index
			ц				Pct	Pct					Pct	
919—Muscatine silt loam, sandy substratum, 0 to 2 percent slopes														
Muscatine, sandy substratum	95	95 C/D	2-0	Silty clay loam, silt loam	MH, ML	A-7-6, A-7-5	0-0-0	0-0-0	100-100	100-100	96-99-1 00	92-98-1 00	43-47 -57	16-18-2 4
			7-16	Silty clay loam, silt loam	MH, ML	A-7-6, A-7-5	0-0-0	0-0-0	100-100	100-100	96-99-1 00	92-98-1 00	43-48 -57	16-19-2 4
			16-20	Silty clay loam	CL, CH	A-7-6, A-6 0-0-0		0-0-0	100-100	100-100	96-99-1 00	92-97-1 00	40-48 -53	19-23-2 5
			20-42	Silty clay loam	CF	A-7-6, A-6 0-0-0	0-0-0	0-0-0	100-100	100-100	96-100- 100	92-97-1 00	38-45 -49	19-23-2 5
			42-50	Silty clay loam, silt loam	CF	A-6, A-7-6 0-0-0		0-0-0	100-100	100-100	95-100- 100	89-97-1 00	31-36 -46	13-17-2 5
			50-79	Loamy fine sand	SC-SM, SM	A-2-4	0-0-0	0-0-0	100-100 100-100 89-95-1 -100 -100 00	100-100		18-23- 30	16-19 -22	2-4 -6

				Engineeri	ing Properti	Engineering Properties-Clinton County, lowa	County, lo	wa						
Map unit symbol and Pct. of Hydrolo	Pct. of	Hydrolo	Depth	USDA texture	Classif	Classification	Fragments	ents	Percenta	Percentage passing sieve number—	g sieve n	umber—	Liquid	Plasticit
SOIL DATE	unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		y index
			u				Pct	Pct					Pct	
920—Tama silt loam, sandy substratum, 0 to 2 percent slopes														
Tama, sandy substratum	85	O	9-0	Silt loam, silty clay loam	CL, ML	A-7-6, A-6 0-0-0		0-0-0	100-100	100-100	97-100- 100	93-98-1 00	39-42 -49	15-17-2
			6-10	Silt Ioam, silty clay Ioam	CL	A-7-6, A-6 0-0-0		0-0-0	100-100	100-100	97-100- 100	93-98-1 00	37-43 -48	15-19-2 1
			10-14	Silt Ioam, silty clay Ioam	72	A-7-6, A-6 0-0-0		0-0-0	100-100	100-100	97-100- 100	93-98-1 00	37-44 -48	15-19-2 1
			14-18	Silty clay loam, silt loam	ರ	A-7-6, A-6 0-0-0		0-0-0	100-100	100-100	98-100- 100	94-98-1 00	36-44 -47	16-22-2 2
			18-32	Silty clay loam, silt loam	C	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	93-98-1 00	35-45 -49	16-23-2 5
			32-45	Silty clay loam, silt loam	CL	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	93-98-1 00	35-43 -49	16-22-2 5
			45-60	Silty clay loam, silt loam	C	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	94-98-1 00	30-38 -41	13-19-2 1
			62-09	Loamy sand, fine sand, loamy fine sand	SC-SM, SM	A-2-4	0-0-0	0-0-0	100-100	100-100	89-95-1 00	18-23- 30	16-19 -22	2-4 -6

				Engineeri	ing Properti	Engineering Properties-Clinton County, lowa	County, Ic	wa						
Map unit symbol and		Pct. of Hydrolo	Depth	USDA texture	Classif	Classification	Fragments	nents	Percenta	Percentage passing sieve number—	g sieve n	nmber-	Liquid	Plasticit
soli name	map unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200	Ĕ	y index
			иI				Pct	Pct					Pct	
920B—Tama silt loam, sandy substratum, 2 to 5 percent slopes														
Tama, sandy substratum	85	O	9-0	Silt loam, silty clay loam	CL, ML	A-7-6, A-6 0-0-0		0-0-0	100-100	100-100	97-100- 100	93-98-1 00	39-42 -49	15-17-2 1
			6-10	Silt loam, silty clay loam	CL	A-7-6, A-6 0-0-0		0-0-0	100-100	100-100	97-100- 100	93-98-1 00	37-43 -48	15-19-2 1
			10-14	Silt loam, silty clay loam	CL	A-6, A-7-6 0- 0- 0		0-0-0	100-100	100-100	97-100- 100	93-98-1 00	37-44 -48	15-19-2 1
			14-18	Silty clay loam, silt loam	CF	A-7-6, A-6 0-0-0		0-0-0	100-100	100-100	98-100- 100	94-98-1 00	36-44 -47	16-22-2 2
			18-32	Silty clay loam, silt loam	CF	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	93-98-1 00	35-45 -49	16-23-2 5
			32-45	Silty clay loam, silt loam	CF	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	93-98-1 00	35-43 -49	16-22-2 5
			45-60	Silty clay loam, silt loam	CF	A-7-6, A-6	0-0-0	0-0-0	100-100	100-100	97-100- 100	94-98-1 00	30-38 -41	13-19-2 1
			62-09	Loamy sand, fine sand, loamy fine sand	SM, SC- SM	A-2-4	0-0-0	0-0-0	100-100	100-100	89-95-1 00	18-23- 30	16-19 -22	2-4 -6

				Engineeri	ing Properti	Engineering Properties-Clinton County, lowa	County, Ic	wa						
Map unit symbol and	Pct. of	Hydrolo	Depth	USDA texture	Classif	Classification	Fragments	nents	Percenta	ge passin	Percentage passing sieve number—	number—	Liquid	Plasticit
son name	unit	group			Unified	AASHTO	>10 inches	3-10 inches	4	10	40	200		y maex
			III				Pct	Pct					Pct	
1152—Marshan clay loam, 0 to 2 percent slopes, rarely flooded														
Marshan, rarely flooded	12	C/D	0-8	Clay loam, silty clay loam, loam, silt loam	ML, MH	A-7-5, A-6 0- 0- 0	0 -0 -0	0-0-0	100-100	82-96-1 00	71-91-1	57-79- 91	39-49 -56	11-19-2 4
			8-14	Loam, clay loam, silty clay loam, silt loam	MH, CL	A-6, A-7-6, A-7-5	0 -0 -0	0-0-0	100-100	82-96-1 00	72-93-1 00	59-82- 93	32-45 -55	12-19-2 4
			14-18	Loam, clay loam, silty clay loam, silt loam	СН, СL	A-6, A-7-6	0 -0 -0	0-0-0	100-100	83-96-1 00	73-93-1 00	60-82- 93	30-42 -51	12-19-2 5
			18-23	Clay loam, silty clay loam, loam, silt loam	CL	A-6, A-7-6 0- 0- 0	0 -0 -0	0-0-0	100-100	83-96-1 00	73-93-1 00	60-82- 93	29-39 -47	12-19-2 5
			23-30	Sandy loam, clay loam, loam, silt loam	CL, SC	A-6, A-7-6	0 -0 -0	0-0-0	92-97-1 00	84-91-1 00	68-81- 97	45-59- 73	29-35 -42	12-16-2
			30-40	Coarse sand, loamy sand, gravelly sand, sand	SW, SP- SC, SP- SM	A-2-4, A-1-b, A-1-a	0-0-0	0- 1- 3	78-90- 97	39-72- 92	25-49- 65	3-8-14	0-17 -22 NP-5 -6	NP-5 -6
			40-79	Gravelly sand, coarse sand, loamy sand, sand	SW, SP- SC, SP- SM	A-1-b, A-1-a, A-2-4	0-0-0	0- 1- 3	78-90- 97	39-72- 92	25-49- 65	3- 8- 14	0-17 -22	NP-5 -6

Data Source Information

Soil Survey Area: Clinton County, Iowa Survey Area Data: Version 21, Sep 28, 2015

